

FLOODING , AN IMPACT OF CLIMATE CHANGE IN JAMAICA

HYDROLOGICAL MODELS FOR RAINFALL – RUNOFF RELATIONSHIPS.

HYDROLOGICAL MODEL – HEC - HMS

**(Hydrologic Engineering Center- Hydrologic Modeling
Systems)**

FOR RUN-OFF COMPUTATION.

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UNIVERSITY OF THE WEST INDIES
MONA CAMPUS, JAMAICA





OUTLINE

- FLOODING IN JAMAICA, CAUSES AND DAMAGES.
- HYDROLOGICAL MODELS FOR RUNOFF COMPUTATION.
HYDROLOGIC ENGINEERING CENTER- HYDROLOGIC
MODELING SYSTEMS (HEC – HMS) OVERVIEW AND USE.
- IMPACT OF CLIMATE CHANGE ON FLOODING.
- CASE STUDY ON THE HOPE RIVER WATERSHED.

What is Flood?

Flood is an overflow of an expanse of water that submerges land.

It is usually due to the **volume of water** within a body of water, such as a river or lake, exceeding the **total capacity of the body**, and as a result some of the water flows or sits outside of the normal perimeter of the body.



Flooding in Jamaica

Flooding is one of the important natural disasters affecting Jamaica causing loss of life and property.

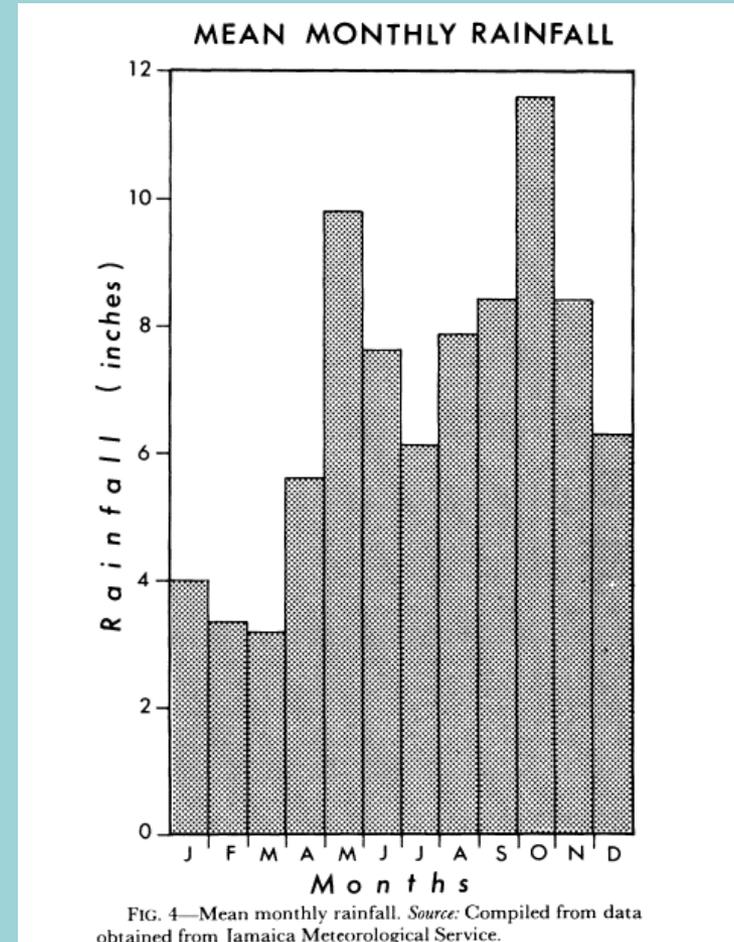
MAIN CAUSES OF FLOODING

- ☞ Rainfall – high intensity rainfall associated with or without tropical storms or hurricane.
- ☞ Topography, Lithology.

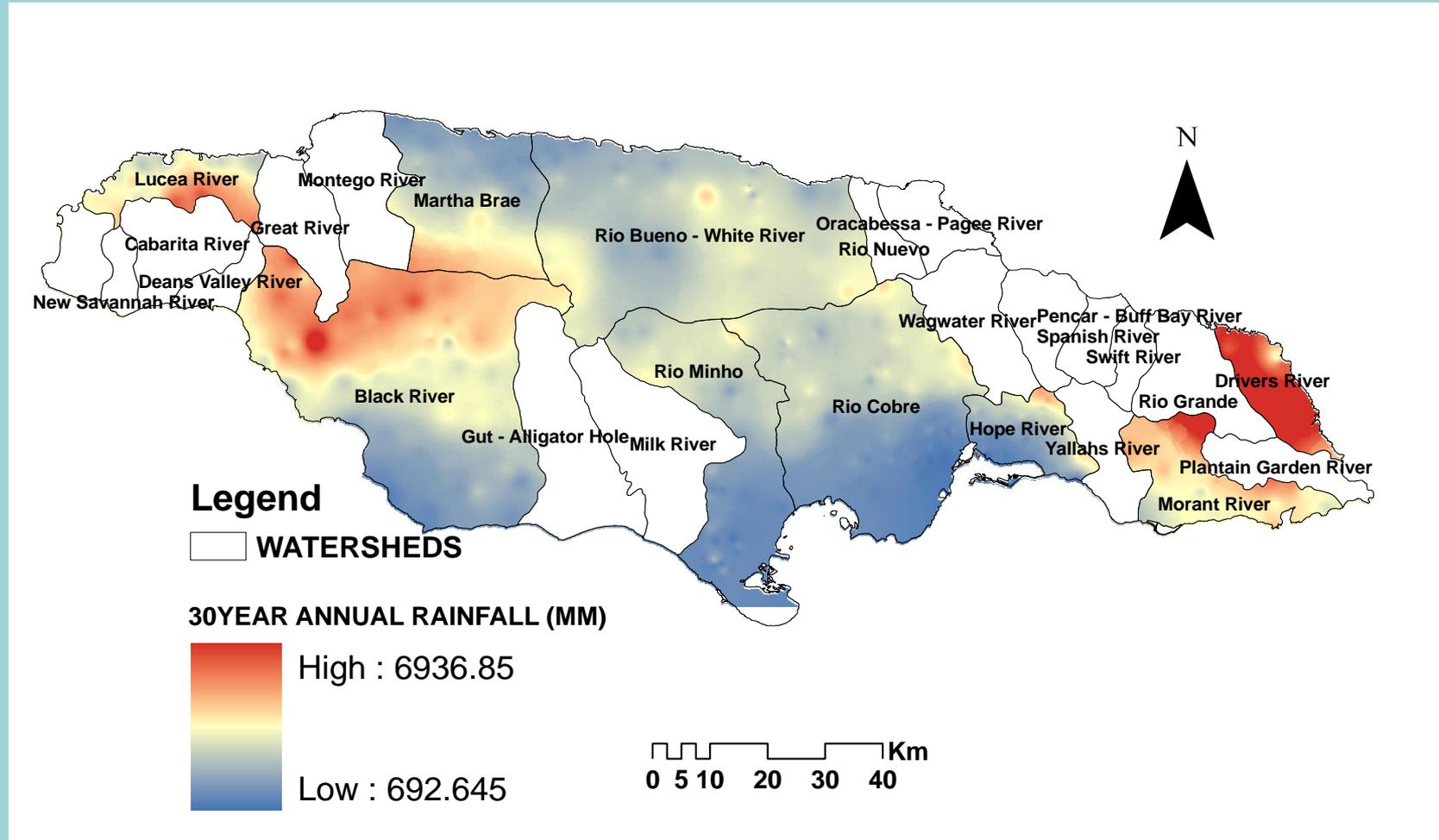
CLIMATE

Jamaica has a tropical maritime (marine) climate. Mean daily temperature ranges from a seasonal low of 26 ° C in February to a high of 28° C in August (33 ° C in recent years).

Islandwide long term mean annual rainfall exhibits a characteristic pattern, with the primary maximum in October and the secondary in May. The main dry season lasts from December to April.

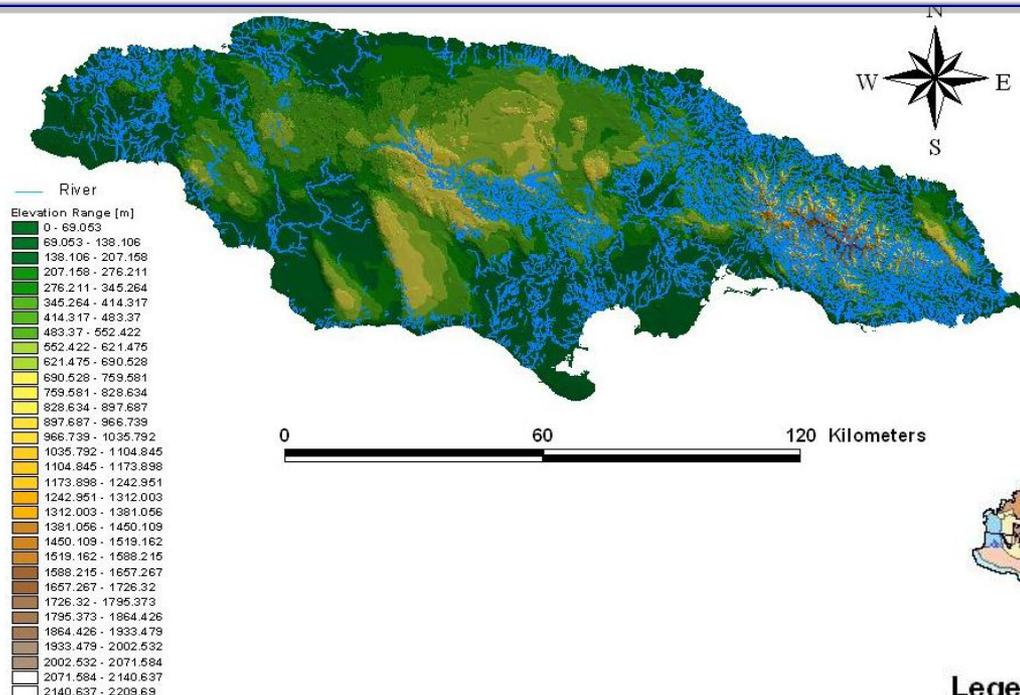


WATERSHEDS IN JAMAICA AND THE 30YR MEAN ANNUAL RAINFALL



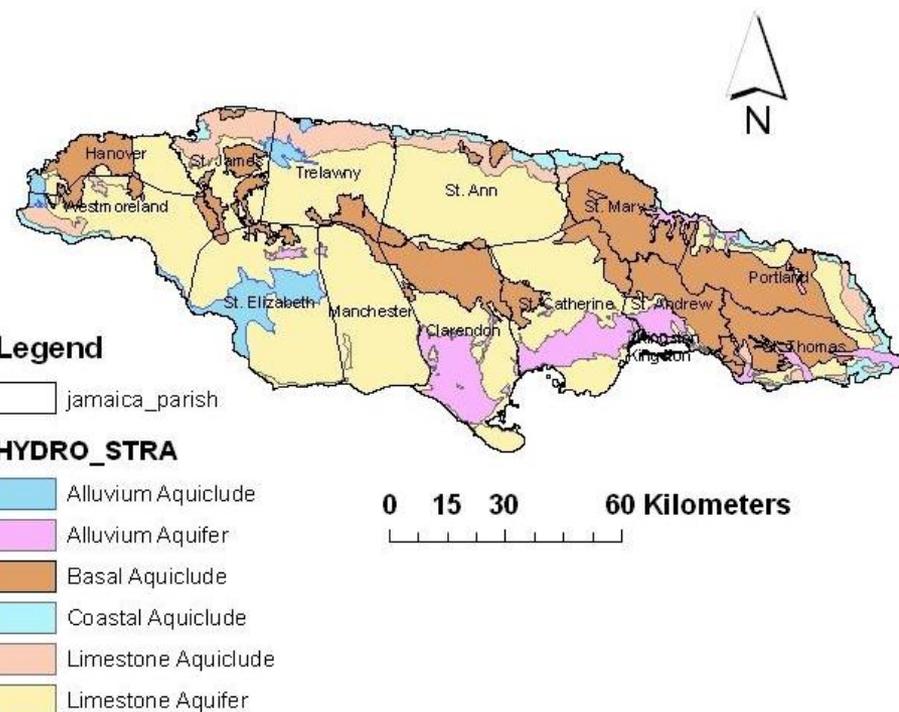
Data: Mona Geoinformatics Ltd. UWI Mona.

TOPOGRAPHY AND HYDROSTRATIGRAPHY OF JAMAICA



DEM of Jamaica showing the rivers.

Hydrostratigraphy of Jamaica with the impermeable basal aquiclude (volcanics, volcanoclastics, shales, conglomerates) occurring in the highlands. Excess rainfall, steep topography leads to high erosion and downslope movement of debris during high rain associated with flooding.



Data: Water Resources Authority, Jamaica

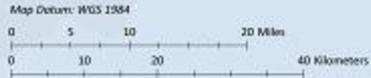
LOCATION OF FLOODING EVENTS AS REPORTED FROM NEWSPAPER ARCHIVES



INLAND FLOODING Jamaica

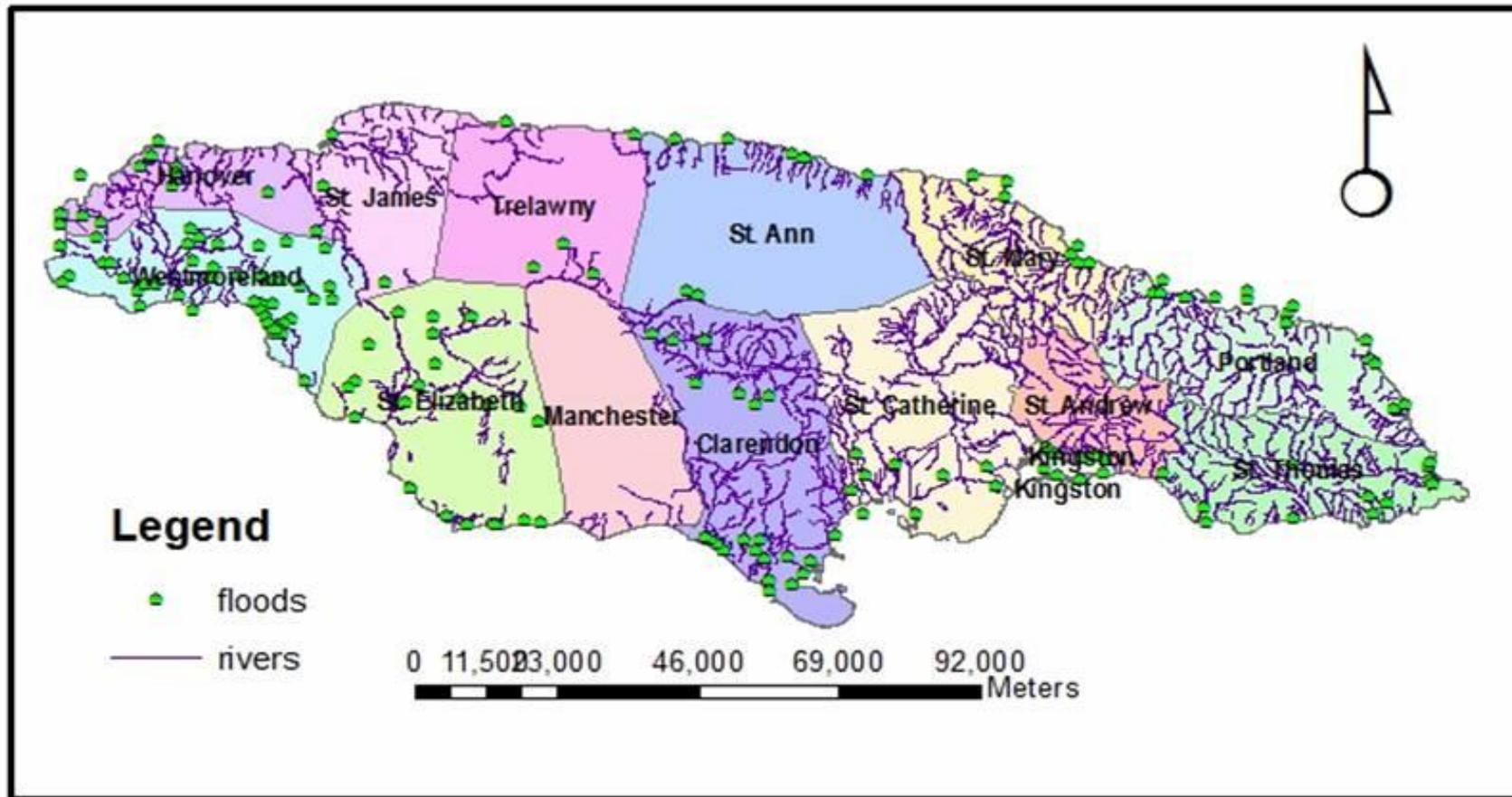
Created by: Mona Geoinformatics Institute
Date: December 2008

- Archived Floods
- Primary Roads
- Secondary Roads
- Mangrove Forest
- Reef
- Drainage
- Water Body
- Urban and Industrial Area
- + Peaks (metres)
- Parish Boundary
- Elevation (metres)**
- 0 - 100
- 101 - 200
- 201 - 300
- 301 - 400
- 401 - 500
- 501 - 600



BAM SOURCES
 - Spot Heights, 2003, Infrared Data
 - Drainage: Water Resources Authority
 - Mangrove Forest and Water Bodies: extracted from the 2006 Land Use GIS dataset, Forestry Department
 - Roads, Topography, village, airports and public stations: produced by Mona Geoinformatics Institute
 - Reports, photographs, text and text: provided by Mona Geoinformatics Institute by means of manual digitization from various sources including satellite imagery and 1982 and 1987 1:50,000 national topographic sheets (National Land Agency)
 Please be aware that the roads represented on this map are only those classed as 'primary' and 'secondary', with some 'other' roads shown as well. 'Other' roads include arterial, residential and collector roads.

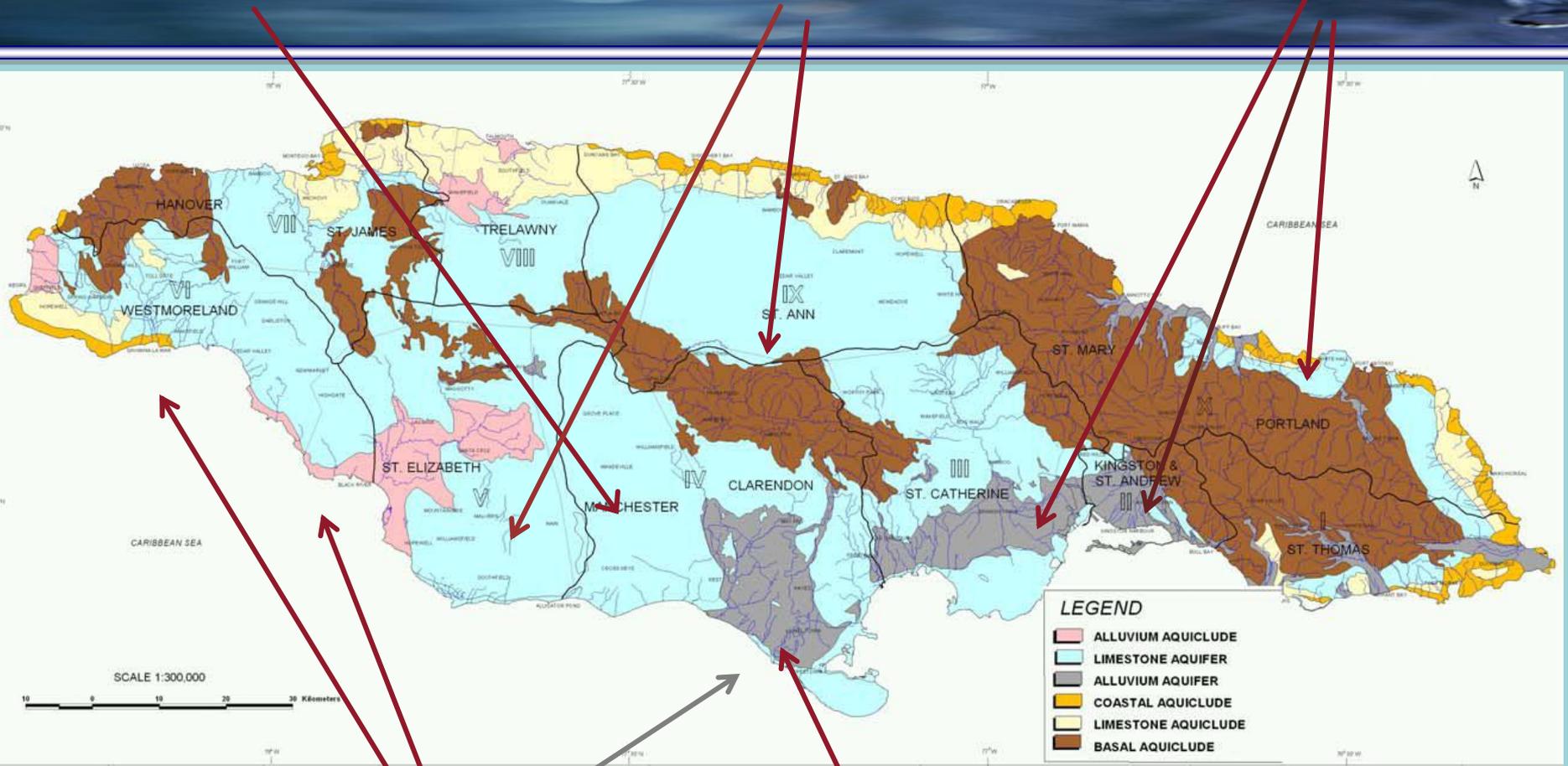
REPORTED FLOOD EVENTS FROM OFFICE OF DISASTER PREPAREDNESS AND EMERGENCY MANAGEMENT (JAMAICA)



Groundwater induced

• Depression

• Riverine



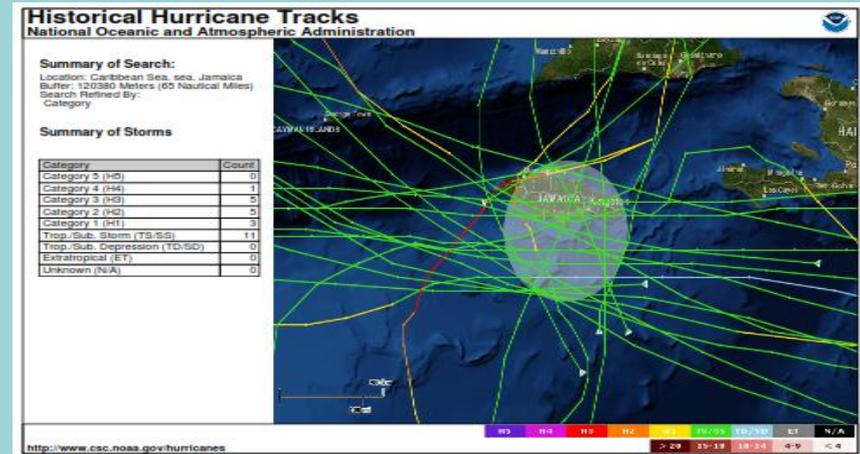
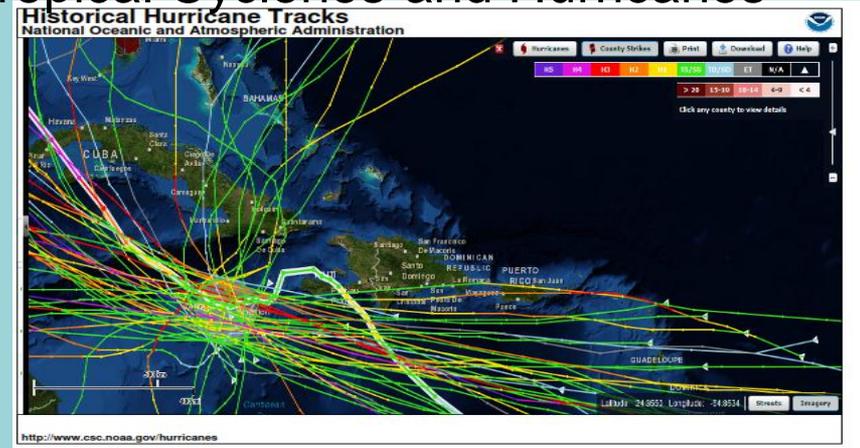
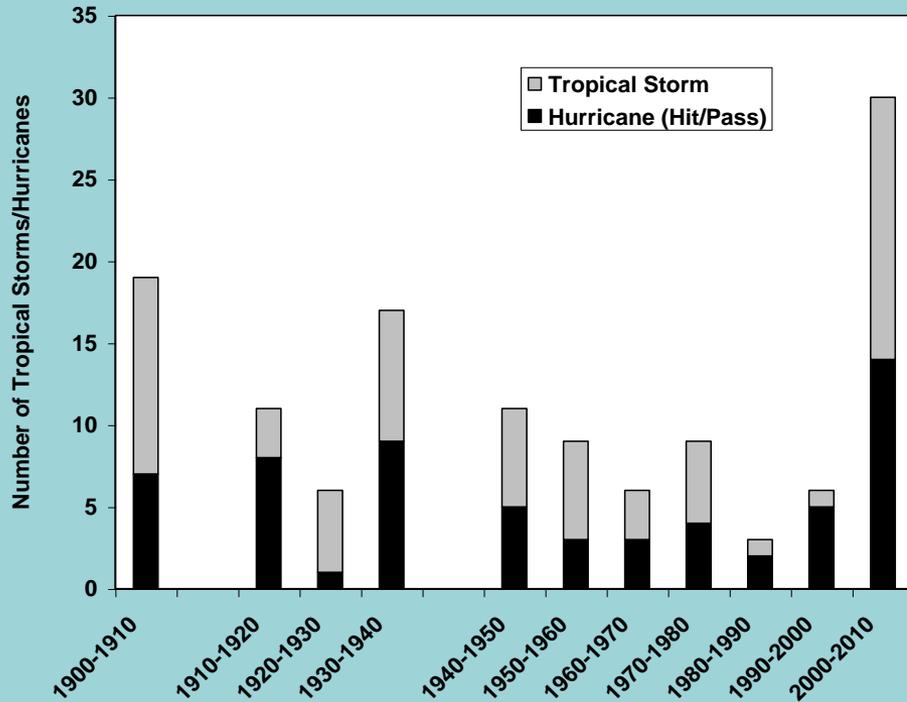
Storm Surge

• Urban Runoff

TYPES OF FLOODING IN JAMAICA

HURRICANES AND TROPICAL STORMS AFFECTING JAMAICA

Tropical Cyclones and Hurricanes



Data from Metservice of Jamaica

Data from 1857-2008, National Hurricane Center, USA

EFFECTS OF FLOODING IN DIFFERENT SECTIONS OF JAMAICA (NEWSPAPER AND INTERNET ARCHIVES)



The remainder of a two storey house which was washed away by the Hope River at River View in Tavern, St Andrew during the passage of Tropical Storm Gustav on August 28, 2008.

Source: Brown (2008), *Jamaica Observer*, October 20

Flooding in Hope River Watershed-Tavern, Kingston > August 28, 2008. Tropical Storm Gustav



Flooding in Gordon Town, Kingston > August 28, 2008. Tropical Storm Gustav



Gordon Town, August 28, 2008. Tropical Storm Gustav



Flooding in Port Maria, Nov 23-24th, 2006 > Cold Front.

FLOODING WOES.....

Bridge over Hope River at Harbour View, Tropical Storm Gustav, August, 2008.



Kintyre, Kingston
September, 2010.
Tropical Storm Nicole.



**BRIDGE AT KINTRYE WHICH
COLLAPSED FOLLOWING
FLOODING , 2007, 2008, 2010**

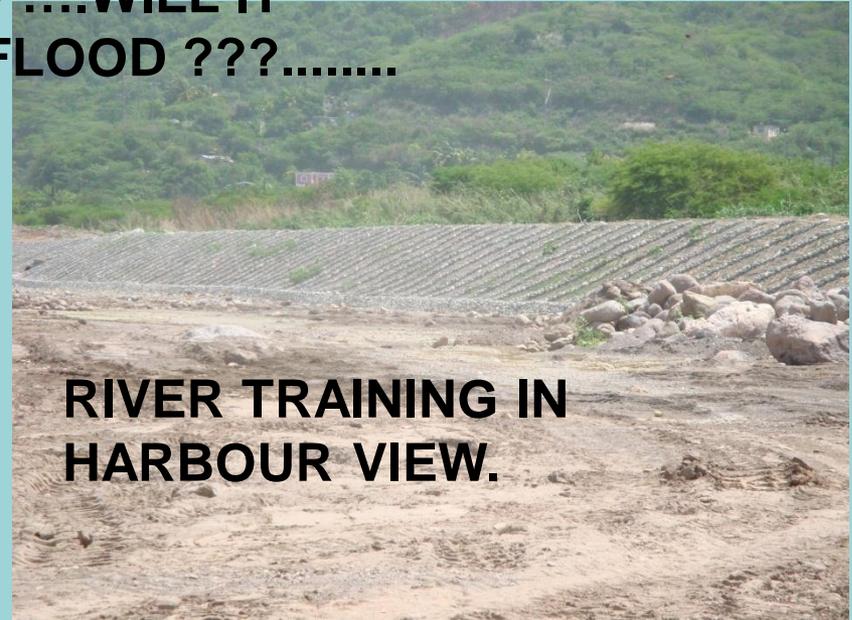


**PRESENT SCENARIOWILL IT
SUSTAIN ANOTHER FLOOD ???.....**



**NEWLY CONSTRUCTED
BRIDGE AT HARBOUR
VIEW. POST NICOLE,
2010**

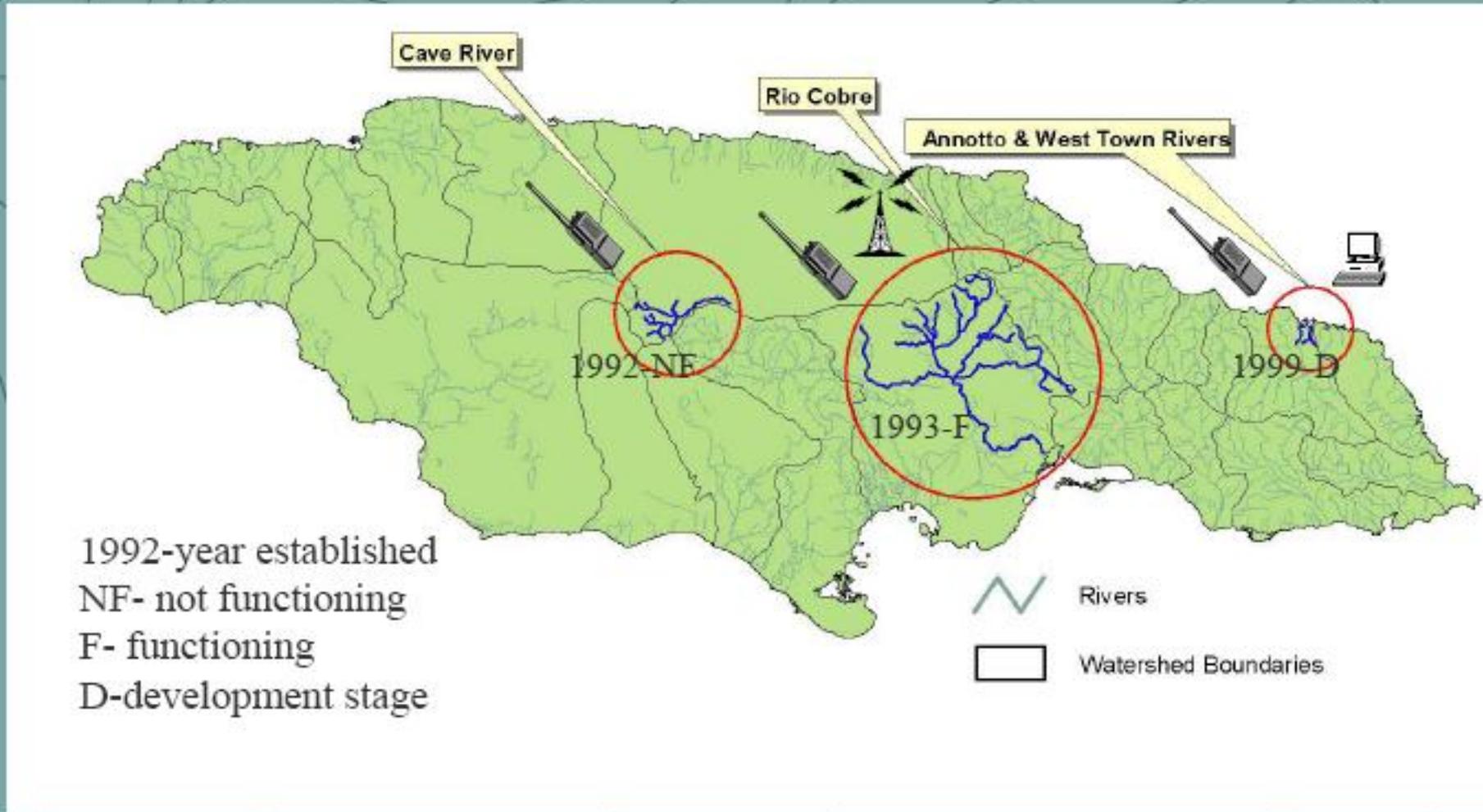
**HOUSE RELOCATED
FOLLOWING FLOODING, 2010**



**RIVER TRAINING IN
HARBOUR VIEW.**

Existing early warning systems in Jamaica

TO BE NOTED : NO EXISTING EARLY WARNING SYSTEM FOR THE HOPE WATERSHED



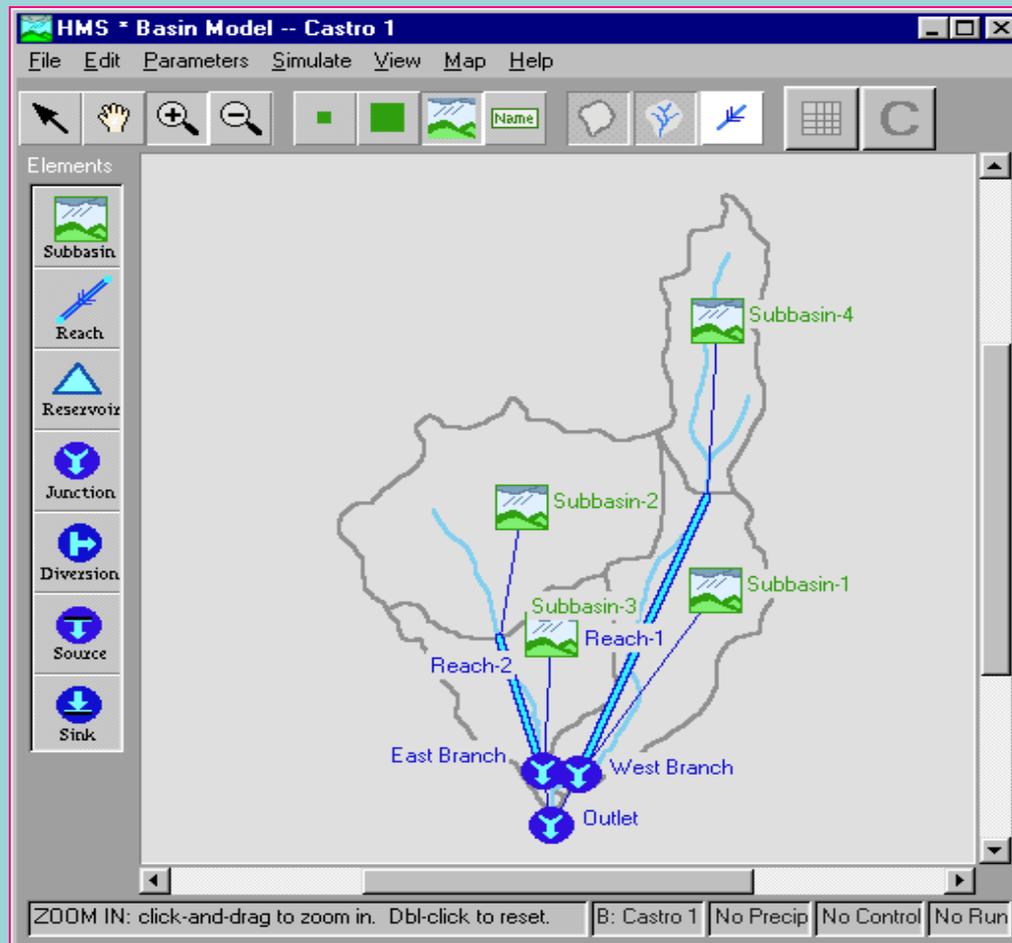
Source : Water Resources Authority of Jamaica

WHAT IS HYDRAULIC AND HYDROLOGIC MODELING

- HEC HMS-(Hydrologic Modeling System) is designed to simulate precipitation- runoff processes of dendritic water systems .
- HEC RAS- (River Analysis System) is a hydraulic model that calculates one-dimensional steady and unsteady flow. The inputs for this model are the hydrograph output from HMS.
- Integration of GIS

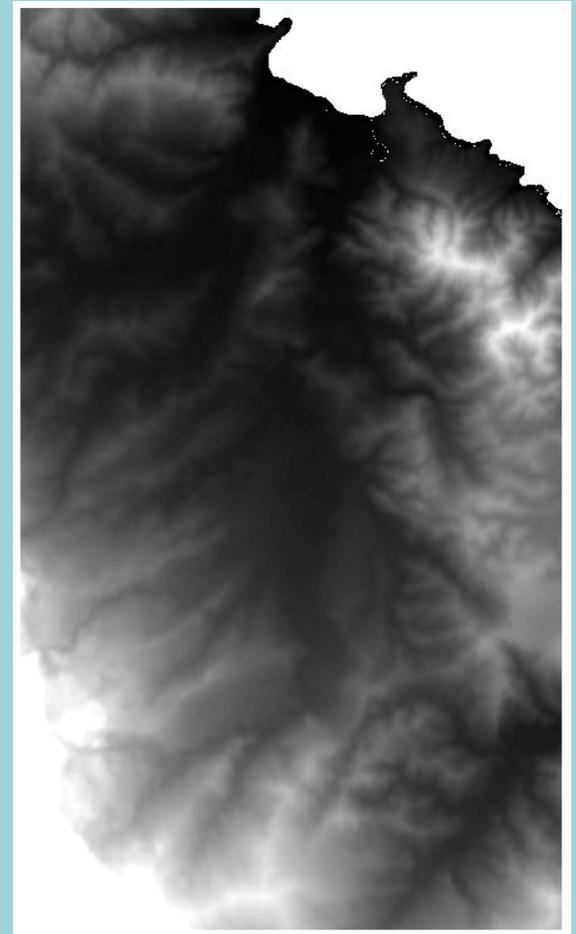
HEC-HMS

*The Hydrologic Engineering Center's
Hydrologic Modeling System (HMS)*



MAIN ROLE: DEM (DIGITAL ELEVATION MODEL)

- Surface shape determines water behaviour
 - characterise surface using DEM
 - slope
 - aspect
 - (altitude)
 - delineate drainage system:
 - catchment boundary (watershed)
 - sub-catchments
 - stream network
 - quantify catchment variables
 - soil moisture, etc.
 - flow times... catchment response



- Other key catchment variables:
 - soils
 - type and association
 - derived characteristics
 - geology
 - type
 - derived characteristics
 - land use
 - vegetation cover
 - management practices
 - artificial drainage
 - storm drains/sewers/ bridges

HEC HMS – Data Structure

1

- METEOROLOGICAL MODEL
- Climatological Data

2

- BASIN MODEL
- Connectivity and Element Data

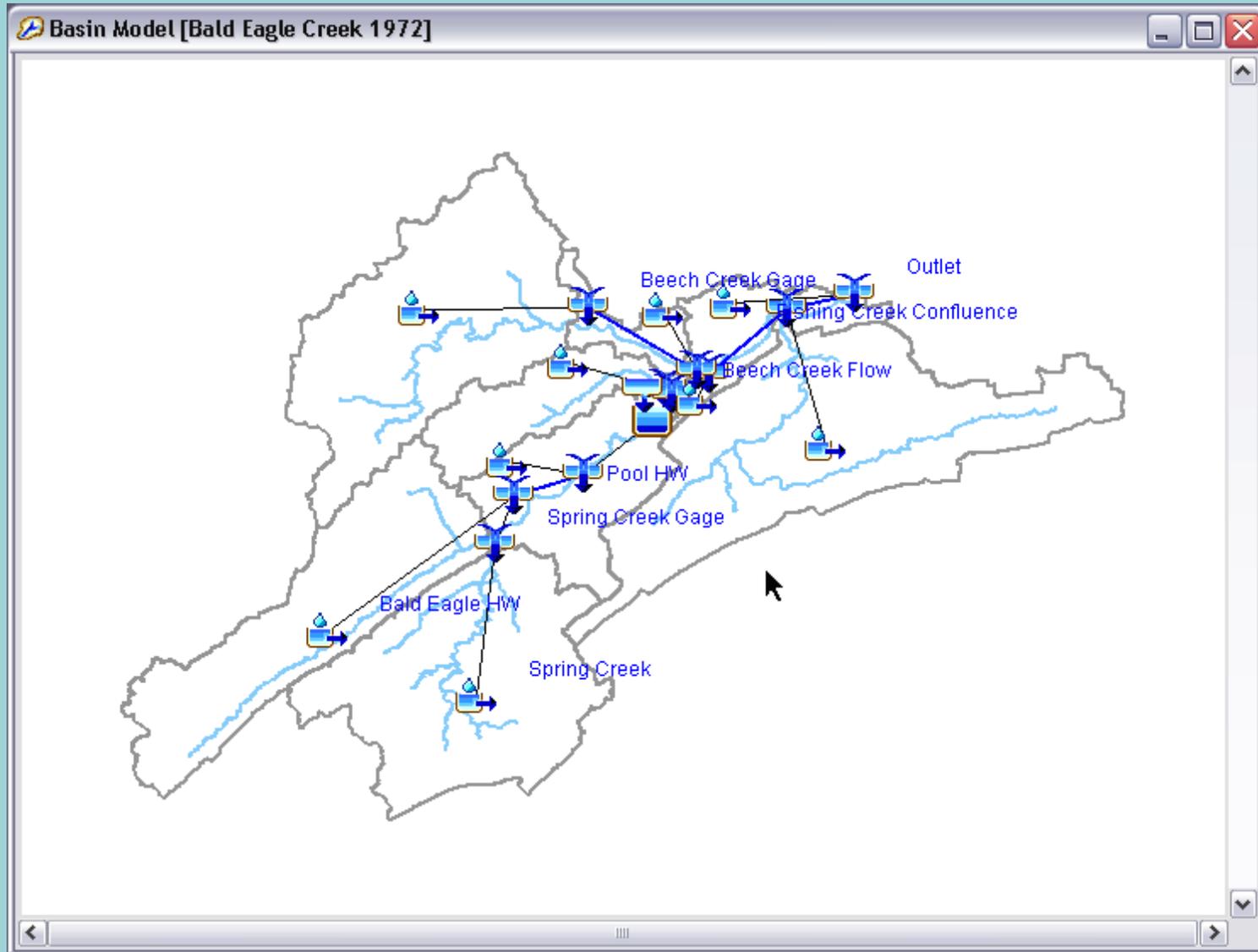
3

- CONTROL SPECIFICATIONS
- Simulation Duration & Time Steps

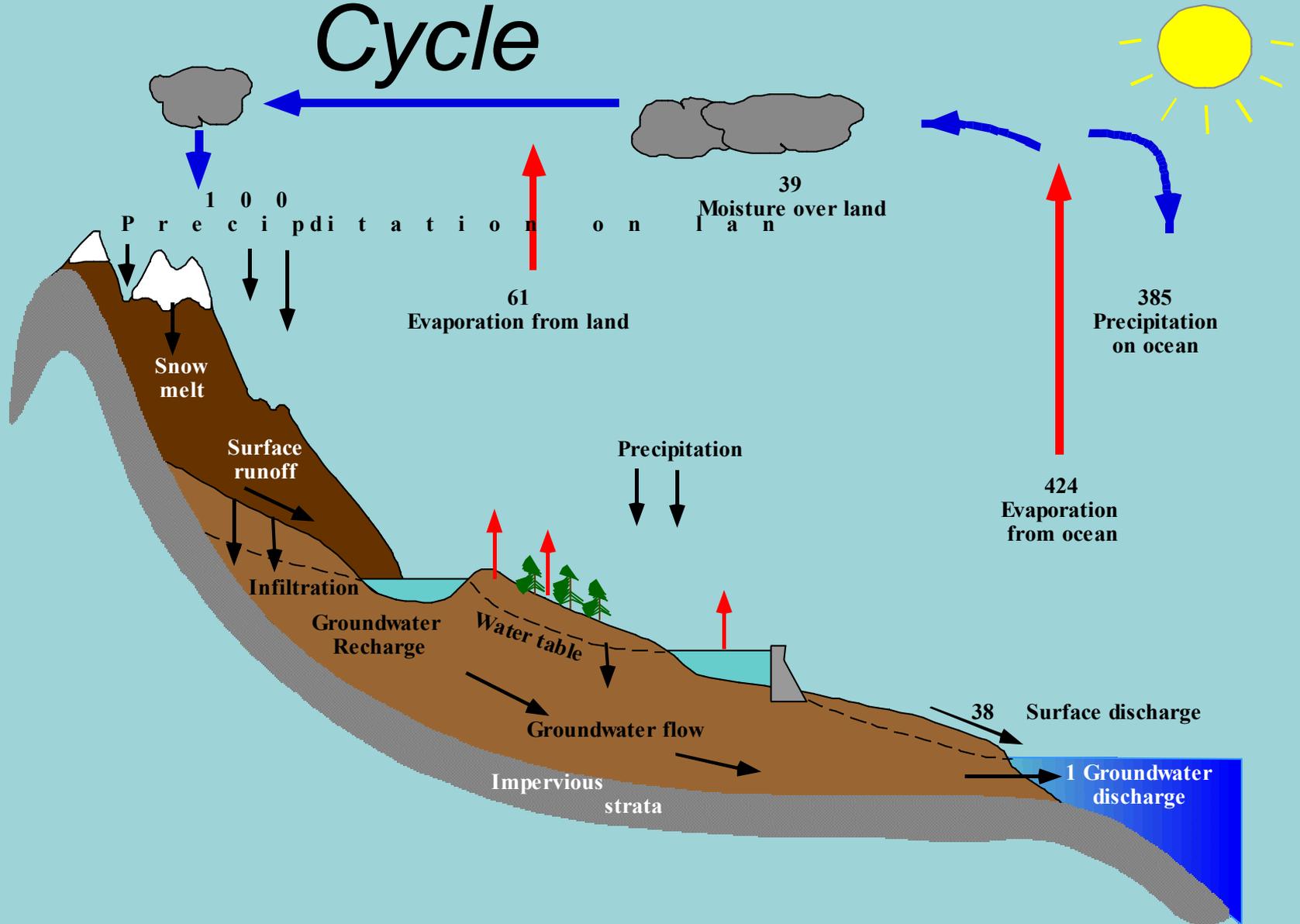
Main Components

- Basin model gives the physical description of the watershed.
 - Subbasin: watershed catchments where rain falls.
 - Reach: rivers and streams.
 - Reservoir: dams and lakes.
 - Junction: confluence.
 - Diversion: bifurcations and withdrawals.
 - Source: springs and other model sinks.
 - Sink: outlets and terminal lakes.
- Meteorologic model describes atmospheric conditions over the watershed land surface.
 - Precipitation.
 - Potential evapotranspiration.
 - Snowmelt.
- Control specifications: Time control during a simulation run

Basin Map



The Hydrologic Cycle



Uses of the HEC Program

Models the rainfall-runoff process in a watershed based on watershed physiographic data

- Offers a variety of modeling options in order to compute Unit Hydrograph for basin areas.
- Offers a variety of options for flood routing along streams.
- Capable of estimating parameters for calibration of each basin based on comparison of computed data to observed data

HEC-HMS Availability

Available Through HEC Vendors

Available at HEC Web Site:

<http://www.wrc-hec.usace.army.mil>

“Public Domain” Program

No Copyright on Software

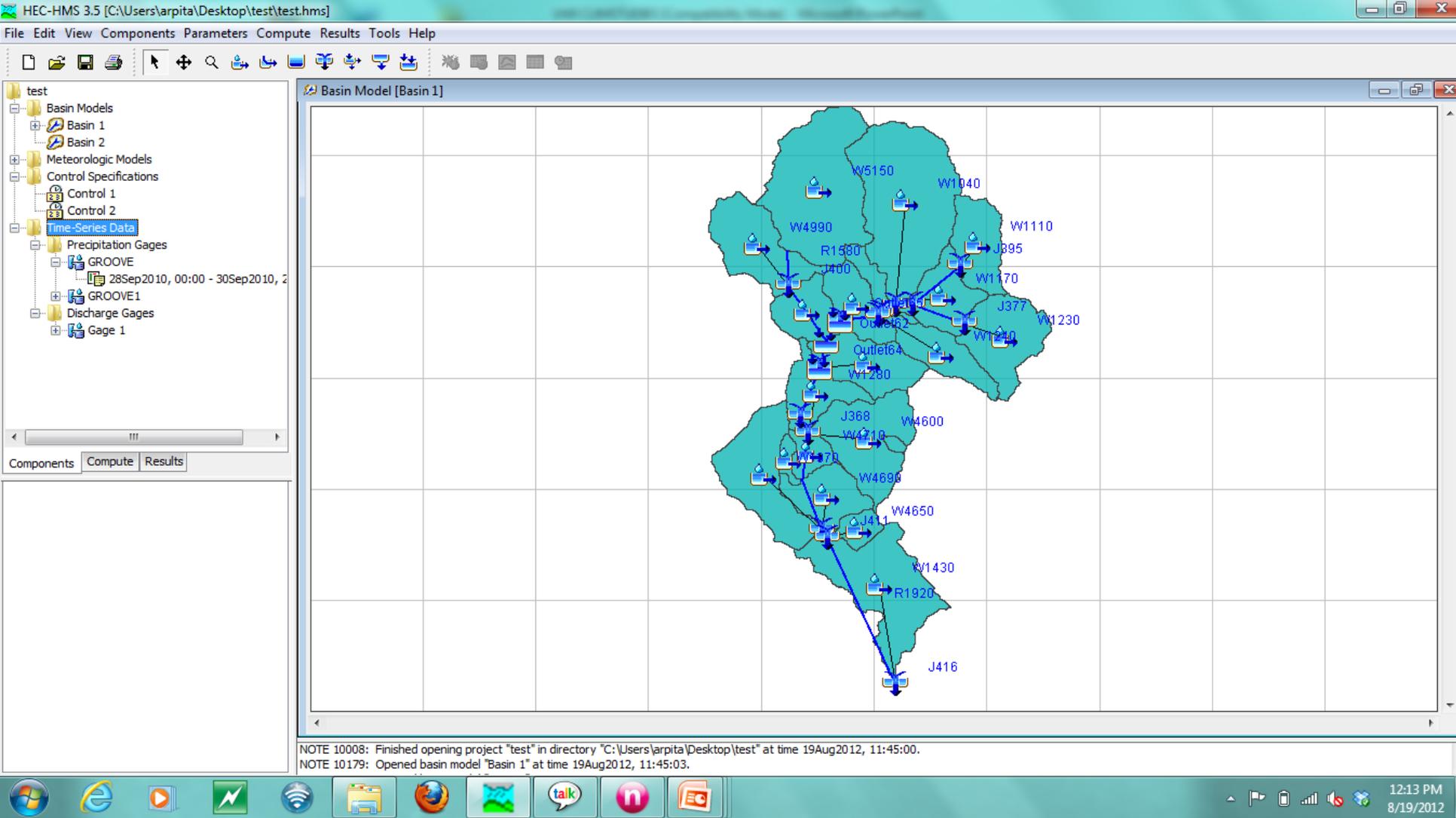
No Copyright on HEC Documentation

Special Training Available

COMPONENTS OF HEC HMS

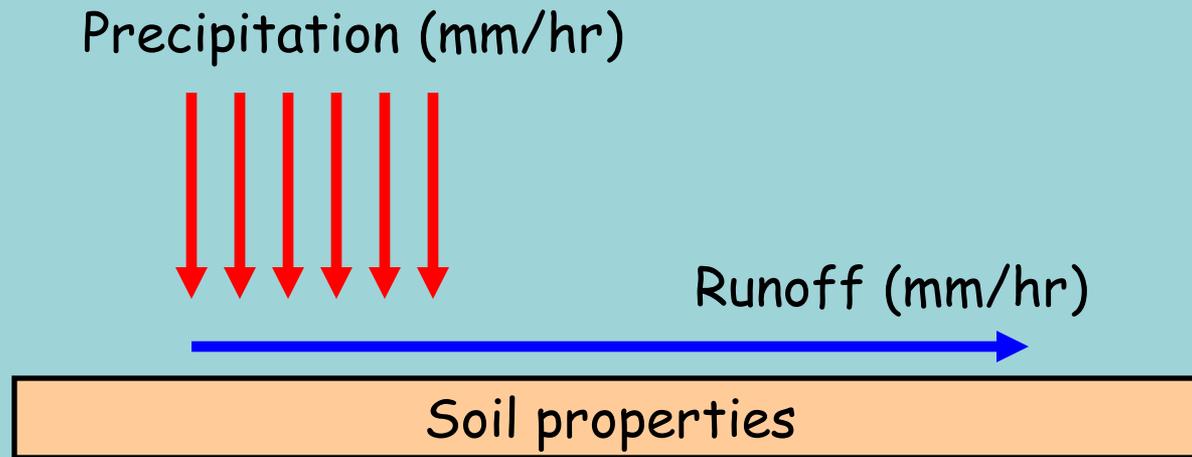
Three components

- *Basin model* - contains the elements of the basin, their connectivity, and runoff parameters
- *Meteorologic Model* - contains the rainfall and evapotranspiration data
- *Control Specifications* - contains the start/stop timing and calculation intervals for the run
- *Time Series data*: component where the actual rainfall , discharge data are entered in tables as per the control specified.



HEC-HMS SCREEN SHOT WITH BASIN MODEL, MET MODEL, CONTROL SPECIFICATION AND TIME SERIES DATA.

Watershed Abstractions



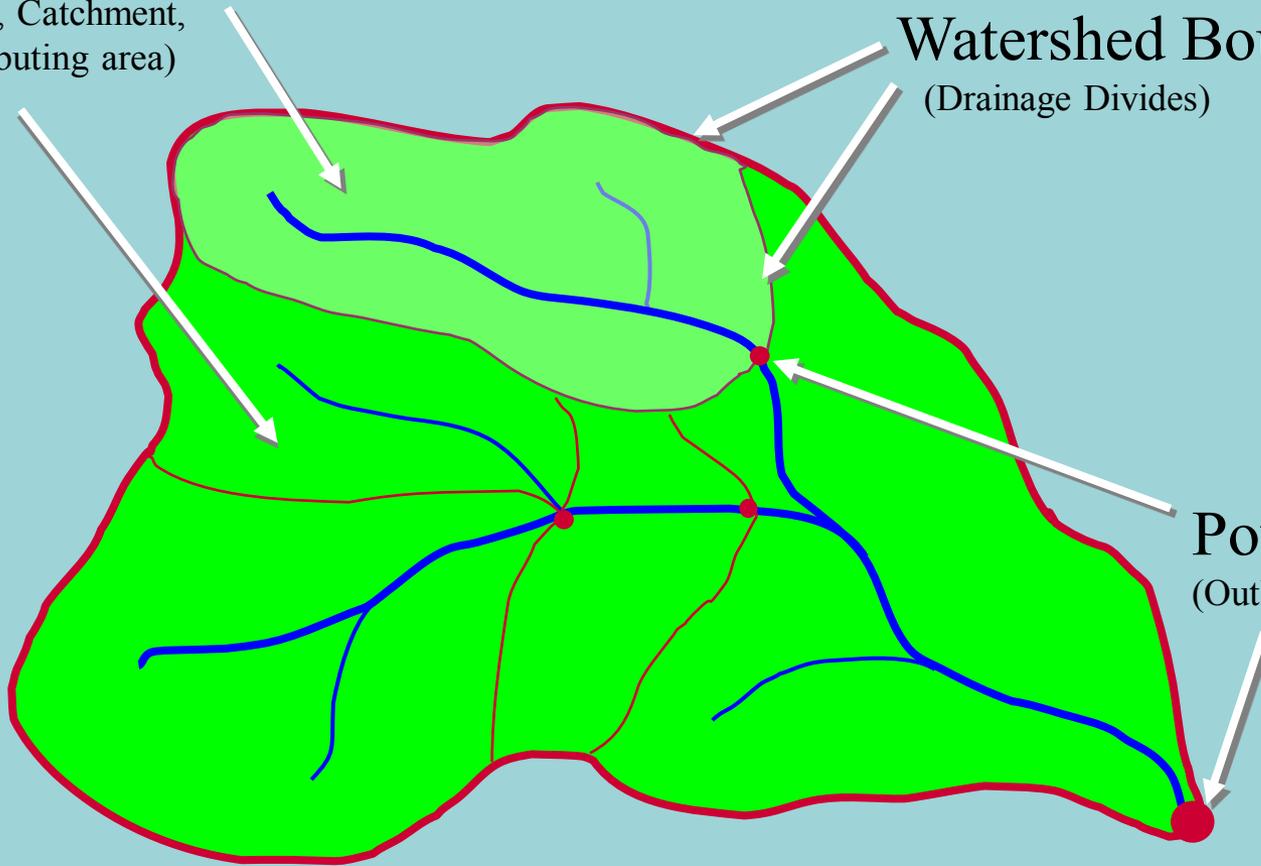
$$\text{Runoff} = f(\text{precipitation, soil properties, moisture conditions})$$

BASICS OF DRAINAGE SYSTEM....

Watershed
(Basin, Catchment,
Contributing area)

Watershed Boundaries
(Drainage Divides)

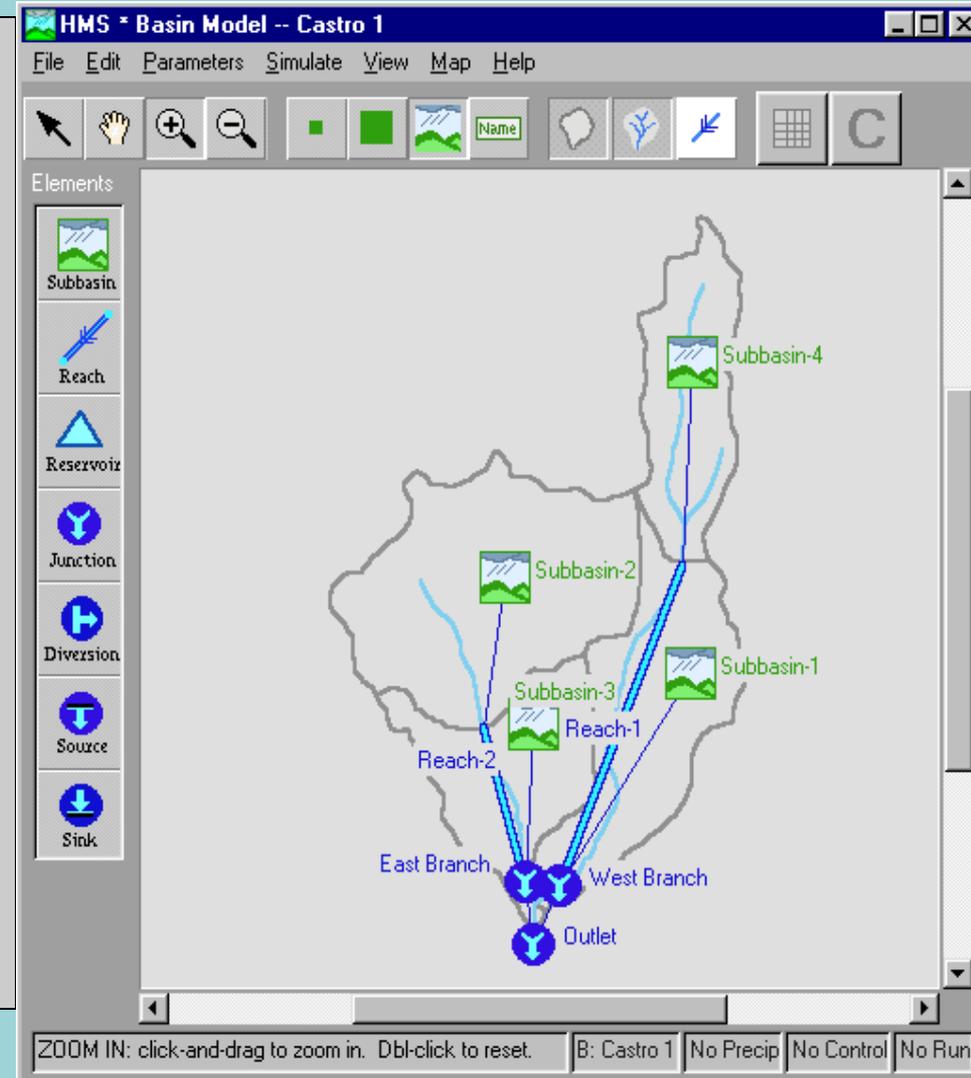
Pour Points
(Outlets)



Basin Model

Basin Model

- Based on Graphical User Interface (GUI)
- Click on elements from left and drag into basin area
- Can import map files from GIS programs to use as background
- Actual locations of elements do not matter, just connectivity and runoff parameters



Basin Model Elements



- **subbasins**- contains data for subbasins (losses, UH transform, and baseflow)



- **reaches**- connects elements together and contains flood routing data



- **junctions**- connection point between elements



- **reservoirs**- stores runoff and releases runoff at a specified rate (storage-discharge relation)

Basin Model Elements



Sink

- **sinks**- has an inflow but no outflow



Source

- **sources**- has an outflow but no inflow



Diversion

- **diversions**- diverts a specified amount of runoff to an element based on a rating curve - used for detention storage elements or overflows

Loss Rate methods

Green & Ampt

Initial & constant

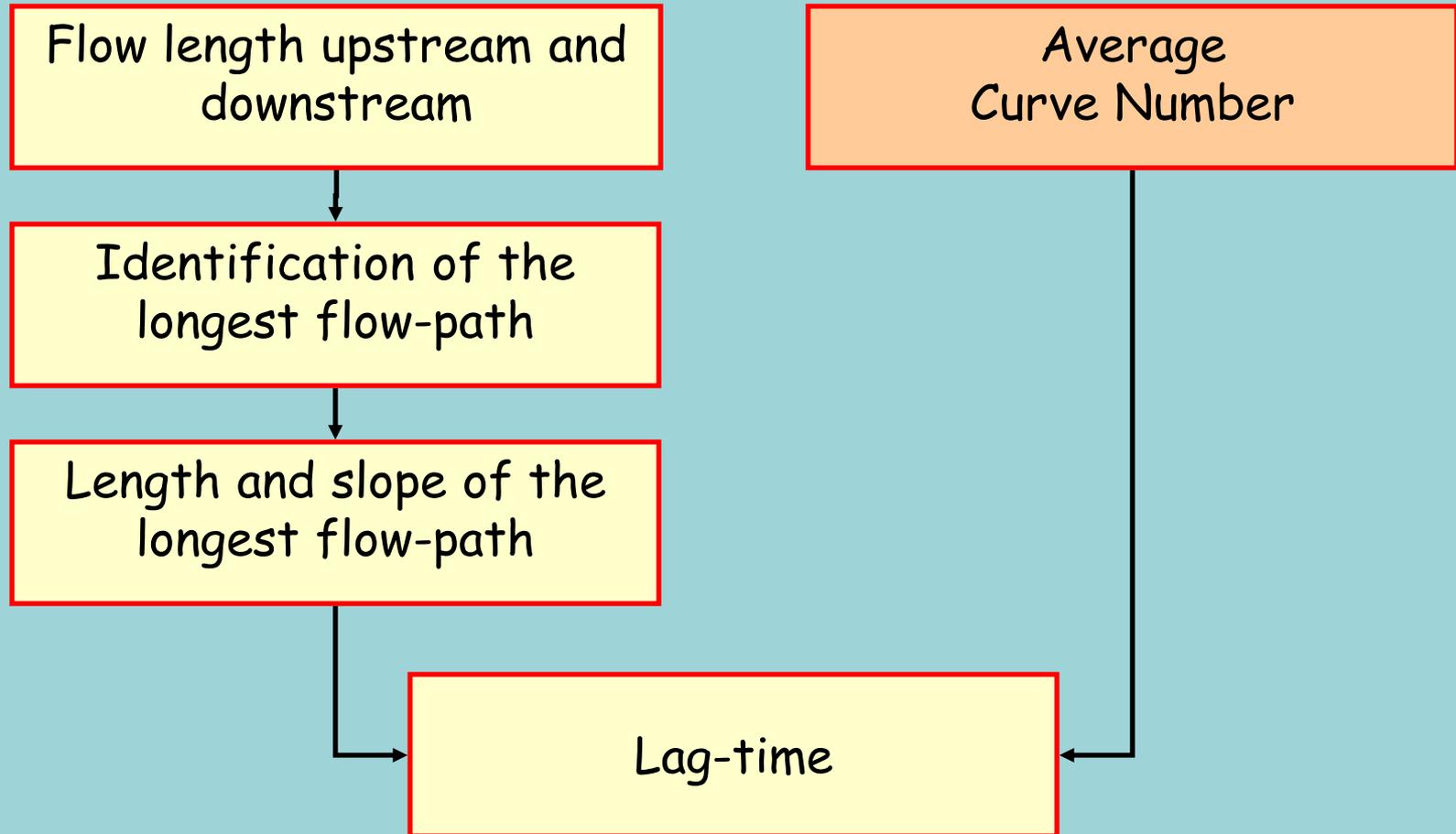
SCS curve no.

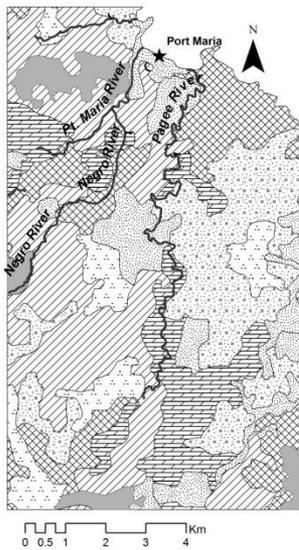
Gridded SCS curve no.

Deficit/Constant

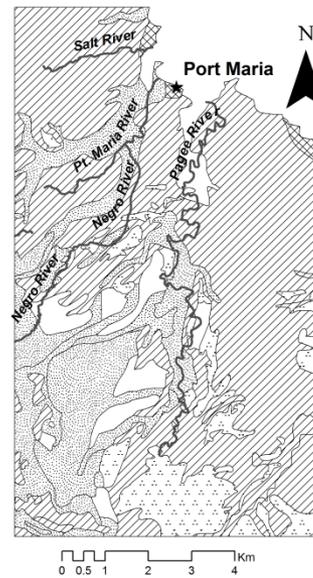
No loss rate

Watershed Lag-Time (SCS)



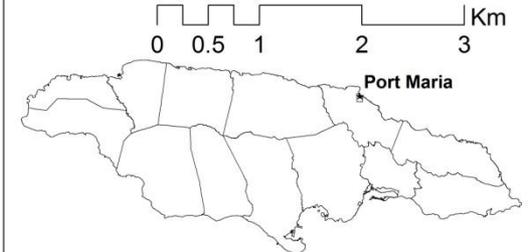
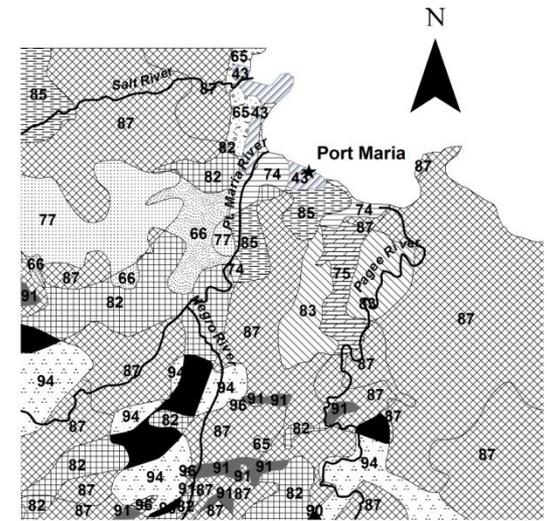


- | | |
|---------------------------------------|---------------------------------------|
| Bamboo and Disturbed Broadleaf Forest | Disturbed Broadleaf Forest and Fields |
| Bamboo and Fields | Fields |
| Buildings and Other Infrastructure | Fields and Disturbed Broadleaf Forest |
| Disturbed Broadleaf | Plantations |



HYDROLOGIC GROUP

- | | | | |
|---|---|---|---|
| A | B | C | D |
|---|---|---|---|



Identifying curve number for catchments using soil, landuse data in ARC GIS

Hydrologic Parameters

- Sub-basin lag-time according to the SCS formula:

$$t_p = \max \left(\frac{L_w^{0.8} [(1000 / CN) - 9]^{0.7}}{31.67 S^{0.5}}, 3.5 \Delta t \right)$$

- t_p : sub-basin lag-time (min)
 L_w : length of sub-basin longest flow-path (ft)
CN: average Curve Number in sub-basin
S: slope of the sub-basin longest flow-path (%)
 Δt : analysis time step (min)

Calculation of Basin Lag time

$$\text{Lag} = \frac{(L^{0.8} * (S+1)^{0.7})}{(1900 * Y^{0.5})}$$

Where: Lag = basin lag time (hours)

L = hydraulic length of the watershed (feet)

$$S = \frac{1000}{\text{CN}} - 10$$

CN values between 50 to 95 are appropriate to this equation.

Y = basin slope (%)

- Reach lag-time for Pure Lag routing:

$$t_{\text{lag}} = \frac{L_s}{60 V_s}$$

t_{lag} : reach lag-time (min)
 L_s : length of reach (m)
 V_s : reach average velocity (m/s)

Baseflow Options

- recession
- constant
monthly
- linear
reservoir
- no baseflow

HMS * Basin Model * Subbasin Editor

Help

Subbasin Name : Subbasin-2 Area (sq. mi.) 4.68

Description : ...

Loss Rate Transform Baseflow Method

Method: No Baseflow

- Recession
- Constant Monthly
- Linear Reservoir
- No Baseflow

OK Apply Cancel

Subbasin name

Stream Flow Routing

- Simulates Movement of Flood Wave Through Stream Reach
- Accounts for Storage and Flow Resistance
- Allows modeling of a watershed with sub-basins

Reach Routing

Flood routing methods:

Simple Lag

Modified Puls

Muskingum

Muskingum Cunge

Kinematic Wave

HMS * Basin Model * Routing Reach

Help

Reach Name : R14

Description : ...

Routing Method : Muskingum

- Lag
- Muskingum
- Modified Puls
- Muskingum Cunge Std.
- Muskingum Cunge 8 Point
- Kinematic Wave
- None

Muskingum K (hr) : 1

Muskingum X : 1

Number of Subreaches : 1

OK Apply Cancel

Baseflow

- Three alternative models for baseflow
 - Constant, monthly-varying flow
 - Exponential recession model
 - Linear-reservoir volume accounting model

Baseflow

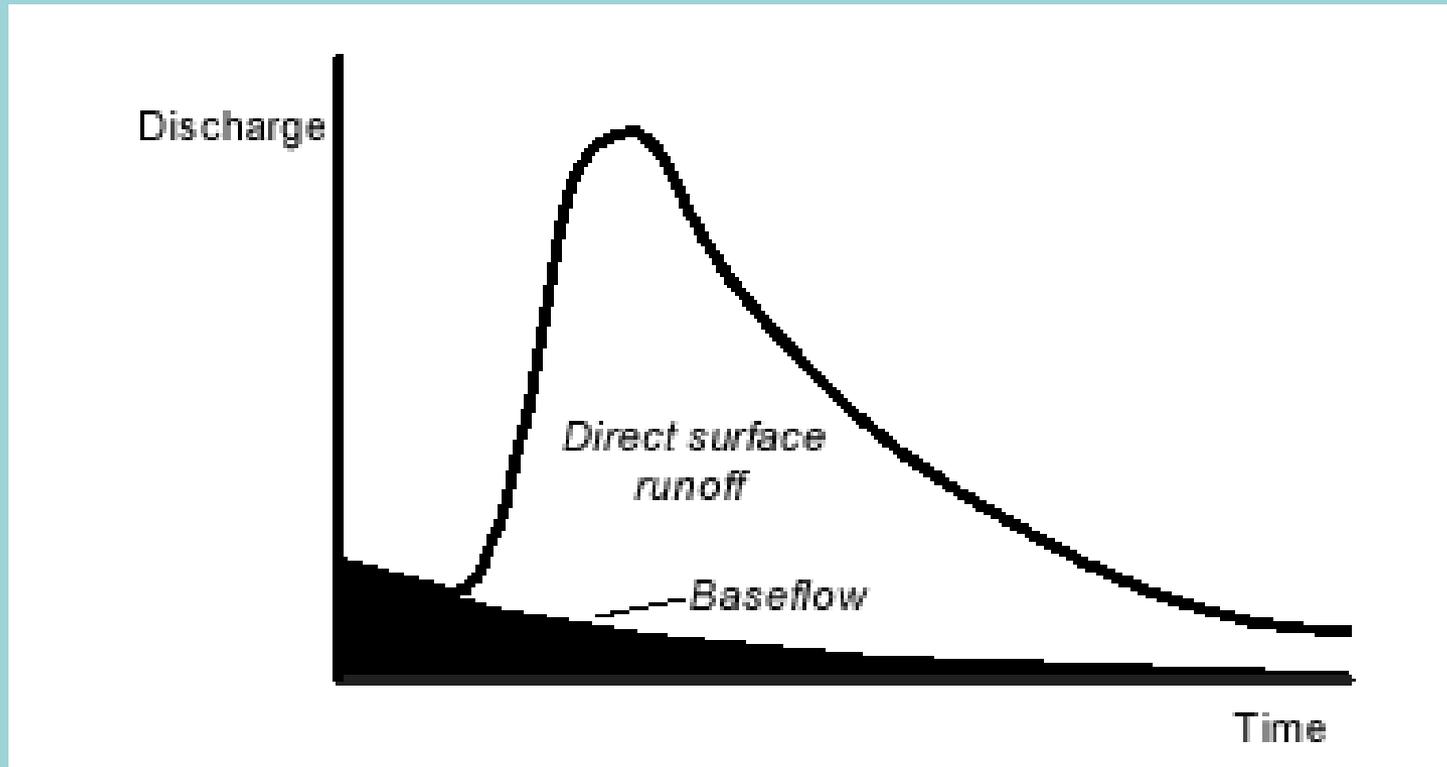
- Exponential recession model
 - Defines relationship of Q_t (baseflow at time t) to an initial value of baseflow (Q_0) as:

$$Q_t = Q_0 K^t$$

- K is an exponential decay constant
 - Defined as ratio of baseflow at time t to baseflow one day earlier
- Q_0 is the average flow before a storm begins

Baseflow

- Exponential recession model



Baseflow

- Exponential recession model
 - Typical values of K
 - 0.95 for Groundwater
 - 0.8 – 0.9 for Interflow
 - 0.3 – 0.8 for Surface Runoff
 - Can also be estimated from gaged flow data

Meteorologic Model

Meteorologic Model

Precipitation

user hyetograph

user gage weighting

inverse-distance gage
weighting

gridded precipitation

frequency storm

standard project storm -
Eastern U.S.

Evapotranspiration-ET

monthly average,

no evapotranspiration

Precipitation

Historical Rainfall Data

Recording Gages

Non-Recording Rainfall Gages

Design Storms

Hypothetical Frequency Storms

Corps Standard Project Storm

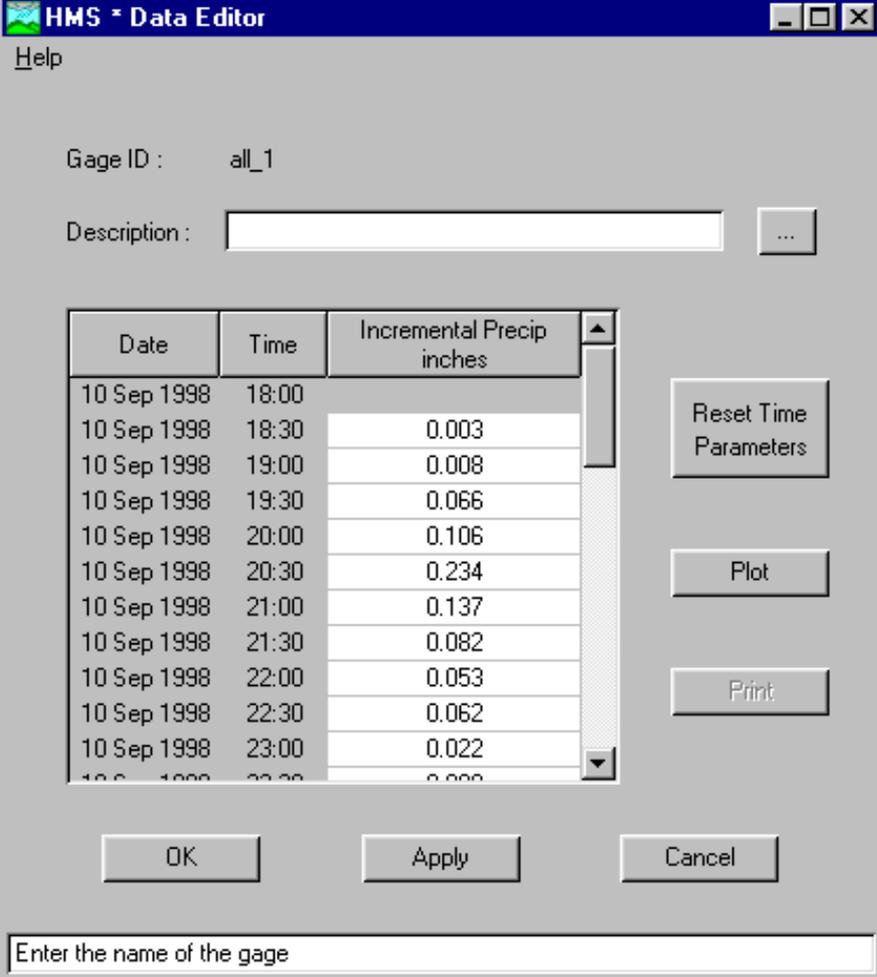
Probable Maximum Precipitation

Gage Data

Gage Data (from project definition screen)

Precipitation gages-
precipitation data for
use with meteorologic
models

Stream gages- observed
level data to compare
computed and actual
results



The screenshot shows the HMS Data Editor window. The title bar reads "HMS * Data Editor". Below the title bar is a "Help" menu. The main area contains the following fields and controls:

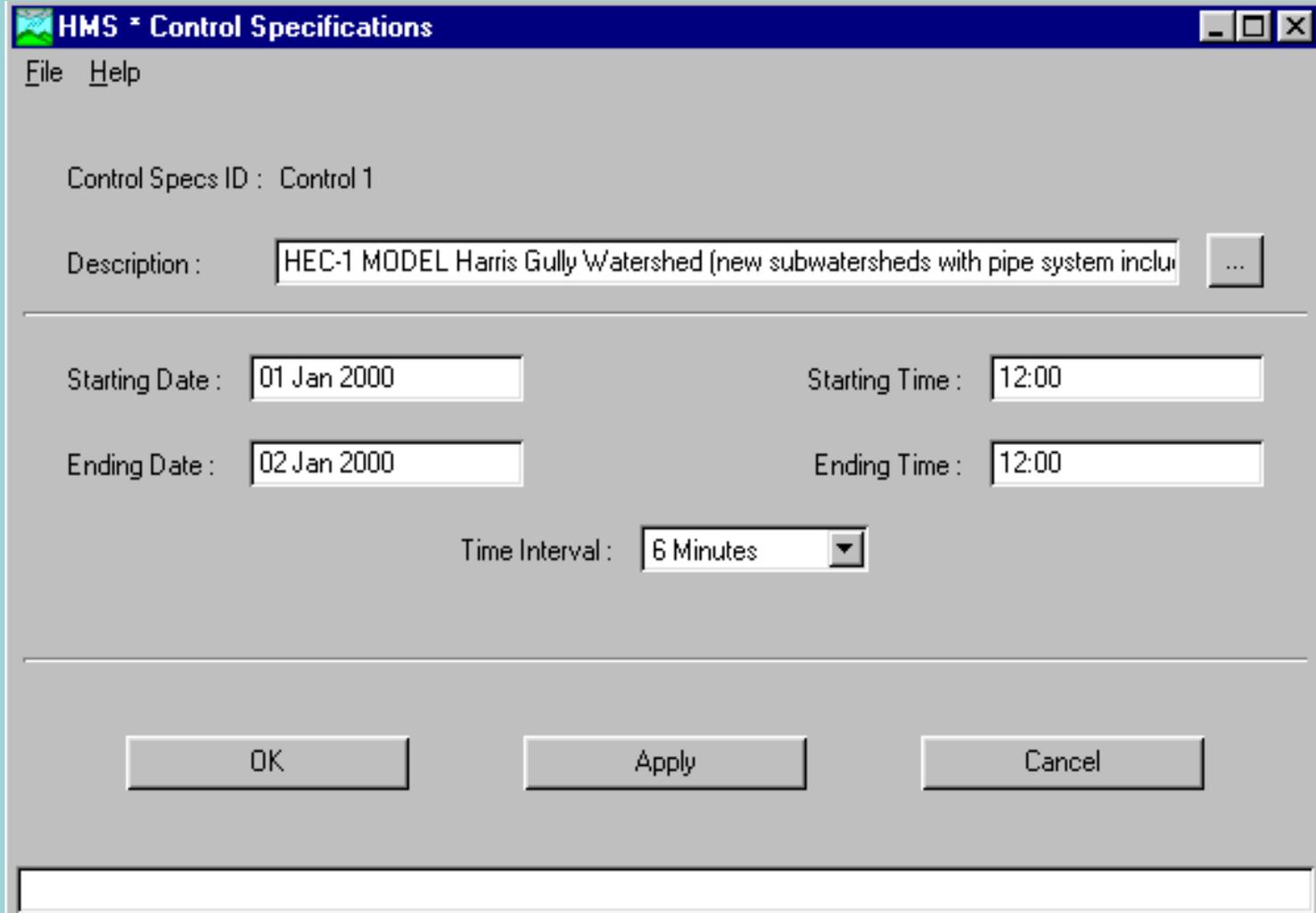
- Gage ID : all_1
- Description : ...
- A table with the following data:

Date	Time	Incremental Precip inches
10 Sep 1998	18:00	
10 Sep 1998	18:30	0.003
10 Sep 1998	19:00	0.008
10 Sep 1998	19:30	0.066
10 Sep 1998	20:00	0.106
10 Sep 1998	20:30	0.234
10 Sep 1998	21:00	0.137
10 Sep 1998	21:30	0.082
10 Sep 1998	22:00	0.053
10 Sep 1998	22:30	0.062
10 Sep 1998	23:00	0.022
10 Sep 1998	23:30	0.000

Below the table are buttons for "Reset Time Parameters", "Plot", and "Print". At the bottom of the window are buttons for "OK", "Apply", and "Cancel". A status bar at the very bottom contains the text "Enter the name of the gage" followed by an input field.

Control Specifications

Control Specifications - Start/Stop/Time Interval



The image shows a Windows-style dialog box titled "HMS * Control Specifications". The window has a menu bar with "File" and "Help". The main content area contains the following fields and controls:

- Control Specs ID : Control 1
- Description :
- Starting Date :
- Starting Time :
- Ending Date :
- Ending Time :
- Time Interval :

At the bottom of the dialog box, there are three buttons: "OK", "Apply", and "Cancel".

Running a project

User selects the

1. Basin model
2. Meteorologic model
3. Control ID for the HMS run

HMS * Run Configuration

File Help

Run ID : Run 1

Description :

Basin ID	
Loss (0.0, 0.00)	
Loss (0.5, 0.05)	BRAYS BAYOU WATERSHED.....HARRIS COUNTY
Loss (1.0,0.10)	BRAYS BAYOU WATERSHED.....HARRIS COUNTY
Loss (0.75,0.075)	BRAYS BAYOU WATERSHED.....HARRIS COUNTY

Met Model ID	Description
100-yr	100-yr rainfall from HCFCF
10-yr	10-yr rainfall from HCFCF
5-yr	5-yr rainfall from HCFCF
25-yr	25-yr rainfall from HCFCF
50-yr	50-yr rainfall from HCFCF

Control ID	
HEC-1 model	BRAYS BAYOU WATERSHED.....HARRIS COUNTY
Sept. 98	
Control 1	

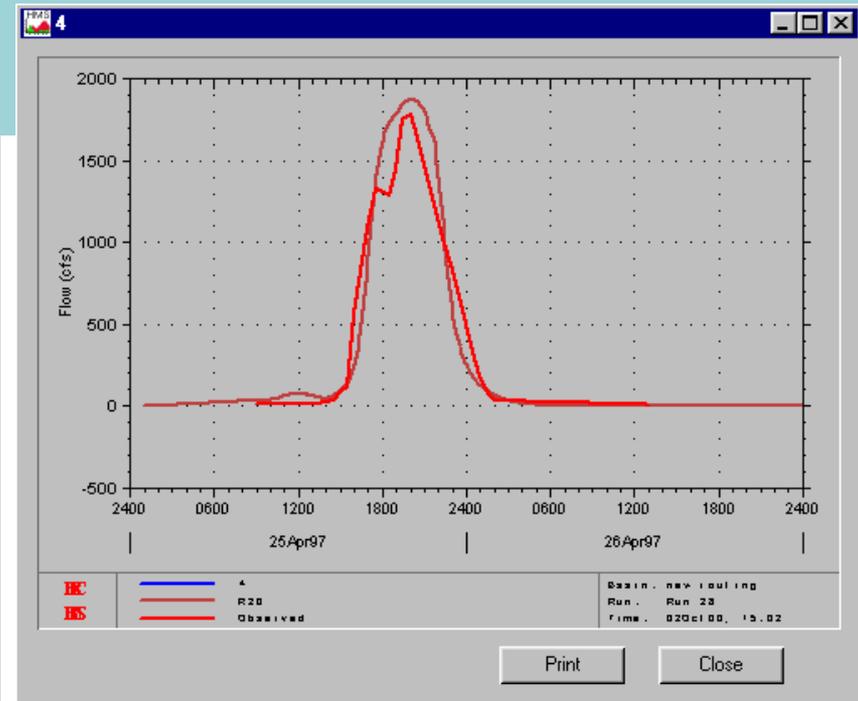
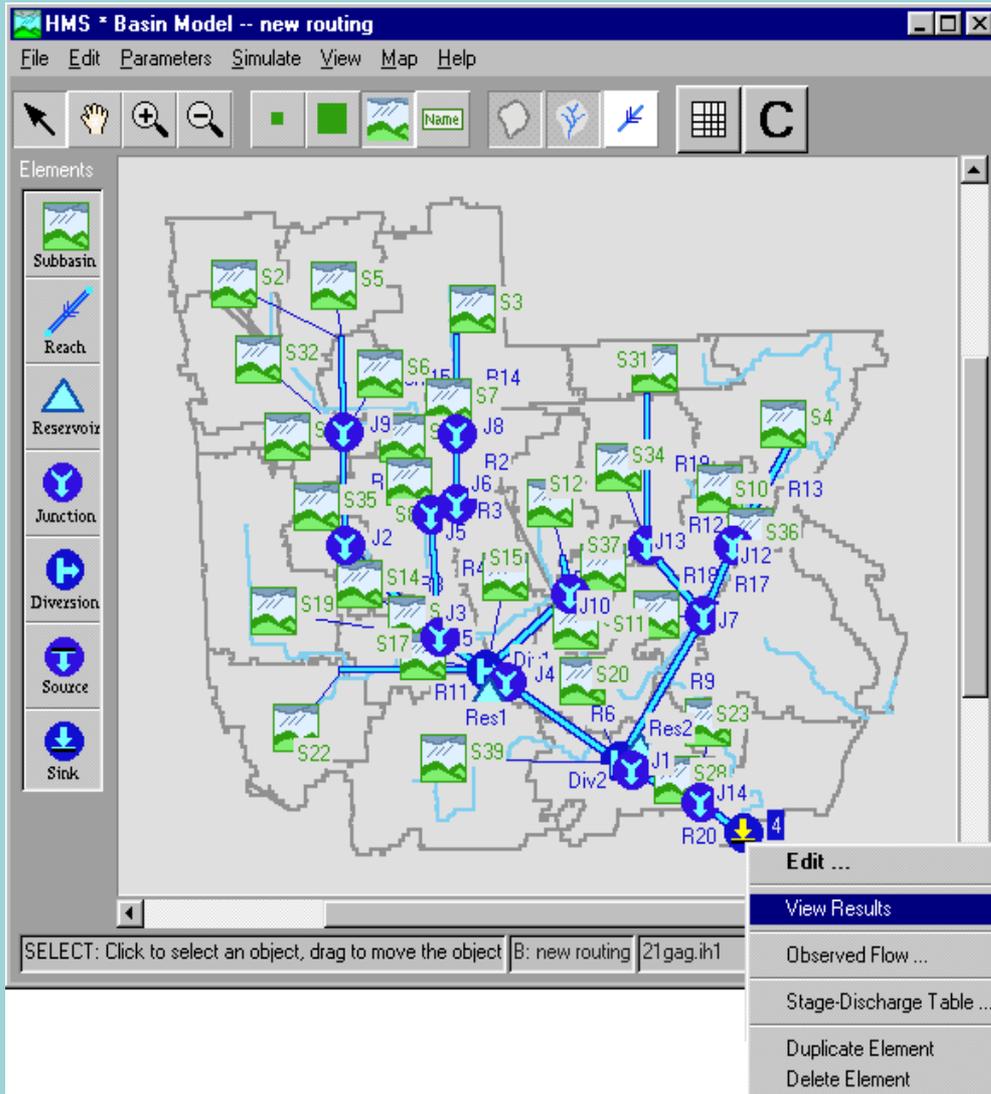
OK Apply Close

Enter a name for this Run.

Viewing Results

- **To view the results:** right-click on any basin element, results will be for that point
- **Display of results:**
 - hydrograph- graphs outflow vs. time
 - summary table- gives the peak flow and time of peak
 - time-series table- tabular form of outflow vs. time
- **Comparing computed and actual results:** plot observed data on the same hydrograph to by selecting a discharge gage for an element

Viewing Results



hydrograph

HEC-HMS Output

1. Tables

- Summary

- Detailed (Time Series)

2. Hyetograph Plots

3. Sub-Basin Hydrograph Plots

4. Routed Hydrograph Plots

5. Combined Hydrograph Plots

6. Recorded Hydrographs - comparison

Viewing Results

HMS * Summary of Results for Sink 4

Project : HG_Basin Run Name : Run 28 Sink : 4

Start of Run : 25Apr97 0106 Basin Model : new routing
End of Run : 26Apr97 2400 Met. Model : APR25a.IH1
Execution Time : 04Oct00 1455 Control Specs : APR25a.IH1

Volume Units : Inches Acre-Feet

Computed Results

Peak Inflow : 1872.8 (cfs) Date/Time of Peak Inflow : 25 Apr 97 2006
Peak Stage : Total Inflow : 3.58 (in)

Observed Hydrograph at Gage : APRIL4

Peak Inflow : 1778.0 (cfs) Date/Time of Peak Inflow : 25 Apr 97 2000
Average Residual : 1.384083e+036 (cfs)
Total Residual : 2.062727e+034 (in)tal Obs. Inflow : -1.268358e+034 (in)

Print Close

Summary table

HMS * Time Series Results for Sink 4

Project : HG_Basin Run Name : Run 28 Sink : 4

Start of Run : 25Apr97 0106 Basin Model : new routing
End of Run : 26Apr97 2400 Met. Model : APR25a.IH1
Execution Time : 04Oct00 1455 Control Specs : APR25a.IH1

Date	Time	Inflow (cfs)	Obs. Q (cfs)	Residual (cfs)
25 Apr 97	1130	71.4	16.3	55.0
25 Apr 97	1136	73.2	16.4	56.8
25 Apr 97	1142	74.7	16.4	58.3
25 Apr 97	1148	75.8	16.5	59.3
25 Apr 97	1154	76.5	16.5	59.9
25 Apr 97	1200	76.7	16.6	60.1
25 Apr 97	1206	76.5	16.6	59.9
25 Apr 97	1212	75.9	16.7	59.2
25 Apr 97	1218	74.9	16.7	58.2
25 Apr 97	1224	73.6	16.8	56.8

Graph Print Close

Time series table

HEC-HMS Output

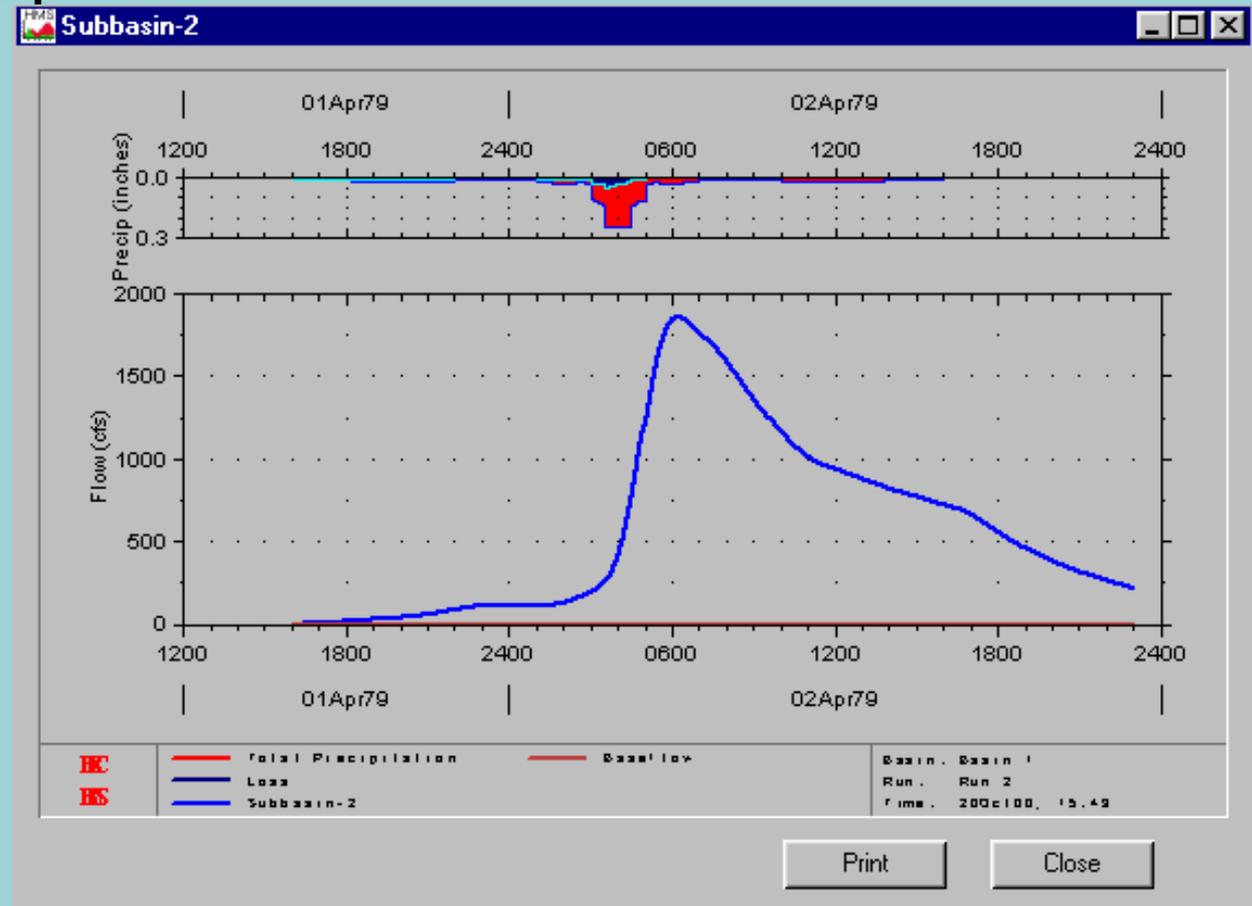
Sub-Basin Plots

Runoff Hydrograph

Hyetograph

Abstractions

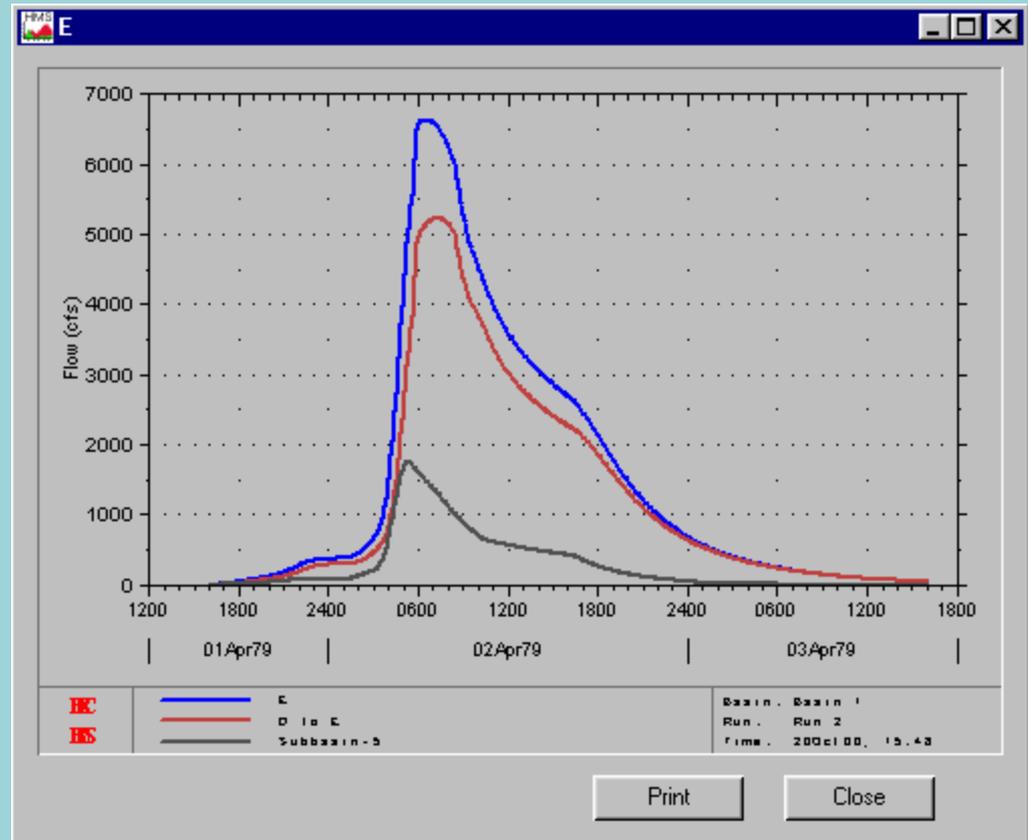
Base Flow



HEC-HMS Output

Junction Plots

Tributary Hydrographs
Combined Hydrograph
Recorded Hydrograph



Purpose of Calibration

Can Compute Sub-Basin Parameters

Loss Function Parameters

Unit Hydrograph Parameters

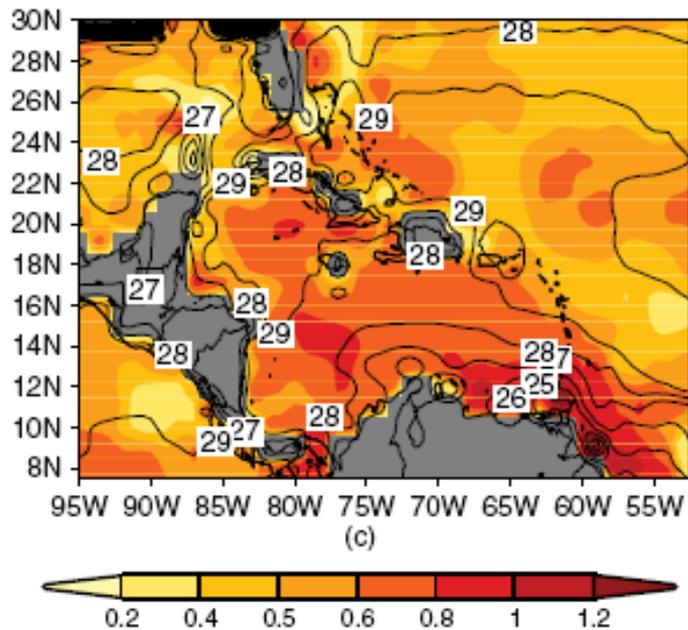
Can Compute Stream Flow Routing
Parameters

Requires Gage Records

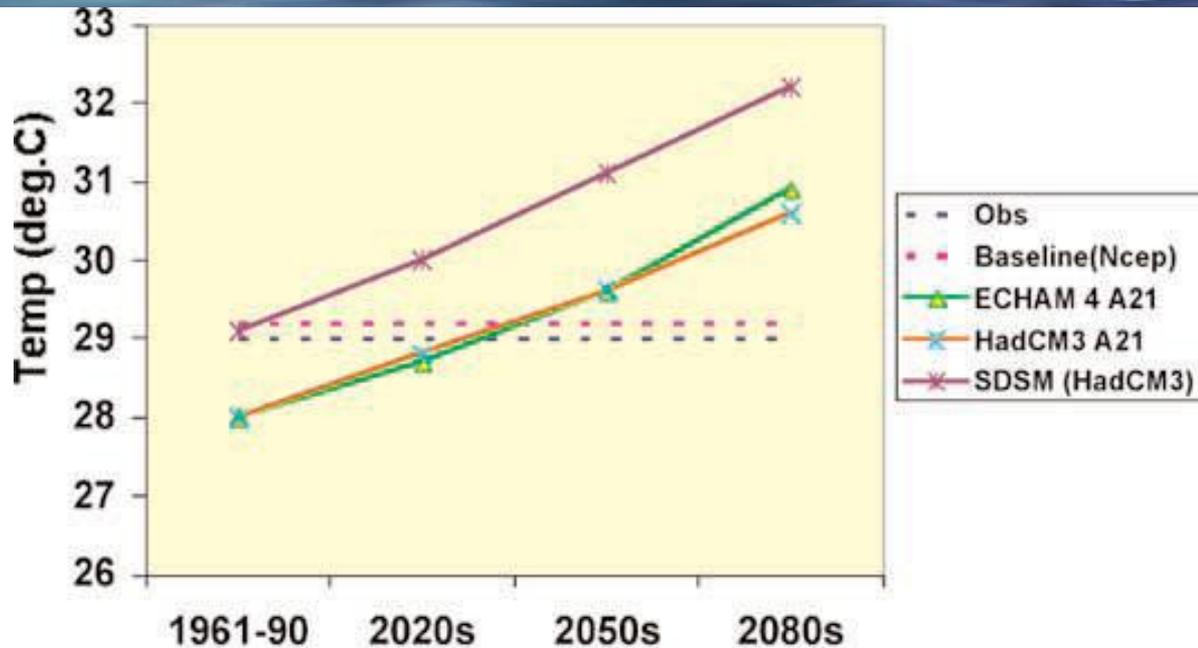
IMPACT OF CLIMATE CHANGE IN THE CARIBBEAN – JAMAICA.

- World's industrial powers (OECD) account for *20% world's population*, but are responsible for *>50 % of global emissions – the cause of global warming and resultant climate change*.
- Developing countries emit *< 25 % of total GHG emissions*.
- Small Island States (SIDS) emit *< 1% of global emissions*.
- SIDS have contributed little to the problem but are among the *most vulnerable* groups to GCC, and have *low adaptive capacity*.
 - Hence adaptation rather than mitigation is most appropriate course

CHANGE IN ANNUAL TEMPERATURE...SEA LEVEL RISE



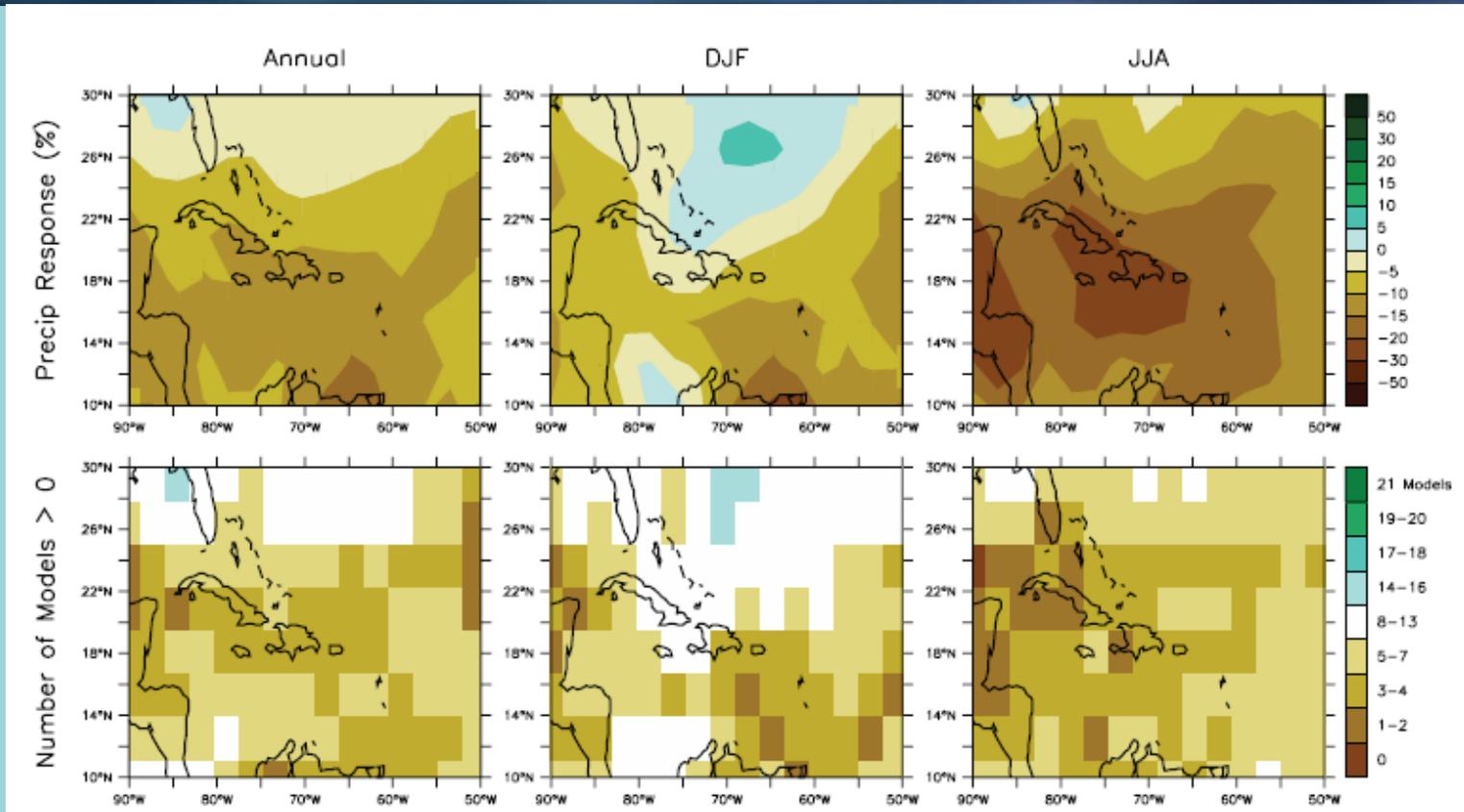
Temperature changes simulated across the Caribbean by Angeles et al., 2006



Observed and baseline (NCEP) temperatures and temperature scenarios at Worthy Park in Jamaica for present (1961-90), 2020s, 2050s and 2080s time slices, obtained by SDSM using HadCM3 with A2 emission scenario. Corresponding results for the Caribbean region given by HadCM3 and ECHAM4 are also shown. (Chen et al. 2008)

Global temperatures have increased by about 0.74°C (0.56°C to 0.92°C) since the 19th century (IPCC, 2007).

CHANGES IN PRECIPITATION...

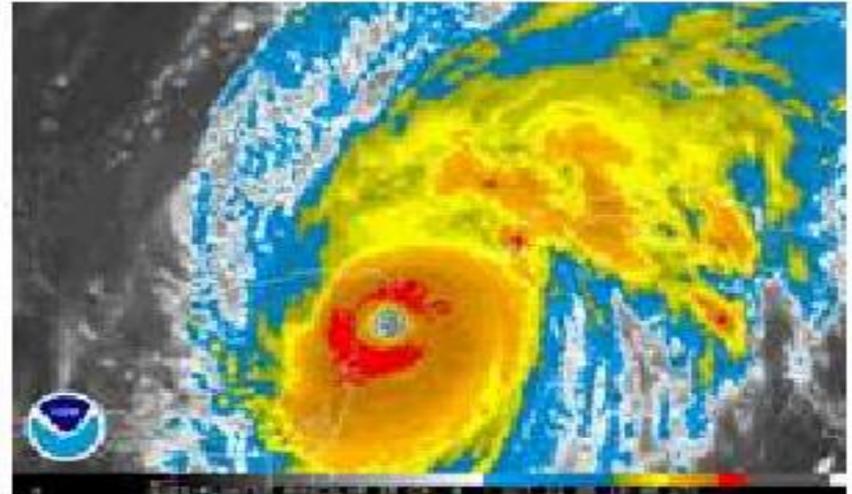


Precipitation changes over the Caribbean from the MMD-A1B simulations. Top row: Annual mean, DJF and JJA fractional precipitation change between 1980 to 1999 and 2080 to 2099, averaged over 21 models. Bottom row: number of models out of 21 that project increases in precipitation (From Christensen et al., 2007).

Hurricanes developing at lower latitudes and becoming more intense in shorter times



Ivan developed near 8°N latitude



Wilma developed from a tropical depression to the most intense category 5 hurricane in less than 24 hrs

6

Effects of increase in temperature ... on small island states.
Increase in frequency of hurricanes.

INCREASED IN FREQUENCY OF HURRICANES AND TROPICAL STORMS WILL LEAD TO AN INCREASE IN FLOODING....

- Flood risk modeling (HADCM2, HADCM3, UKMO, 1999) suggests that by 2080, numbers facing severe floods in the Caribbean, Indian and Pacific Ocean regions would be 200 times higher than if there were no SLR.

Increase in flooding due to high intensity rains from hurricanes and storms will affect the coastal areas of Jamaica.. Ocho Rios, Port Maria, parishes of St. Thomas , Kingston and Portland.

Major damages have been done by tropical storm Gustav and Nicole in Portland, St. Thomas , Kingston and St. Andrew.

Flooding along coastal areas affects tourism as well as the transportation to the major airports.

HOPE RIVER WATERSHED-CASE STUDY

The Hope River supplies water to Kingston and flows through the eastern section of the city and has been responsible for high levels of damage in the past. Damage is usually caused by fast-flowing torrents carrying debris of varying size.

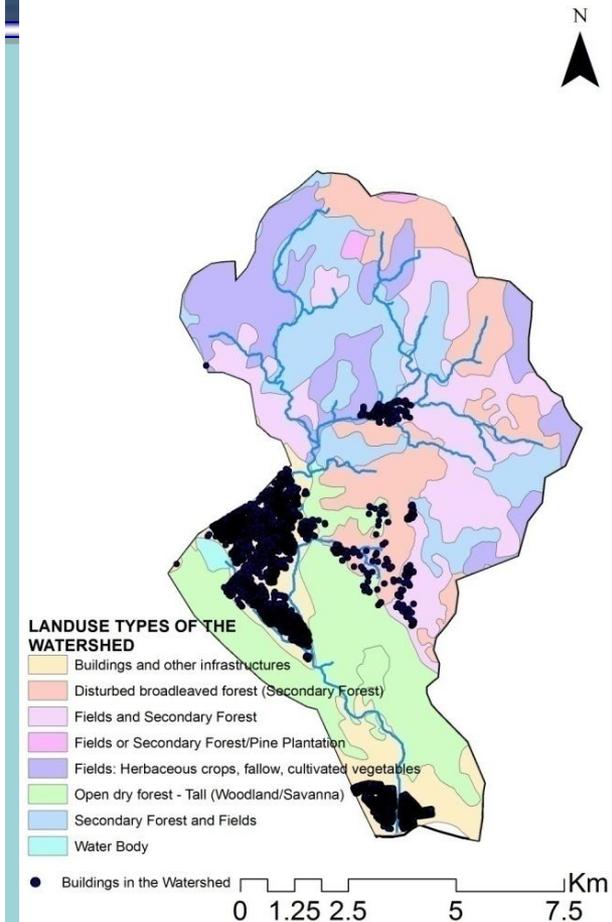
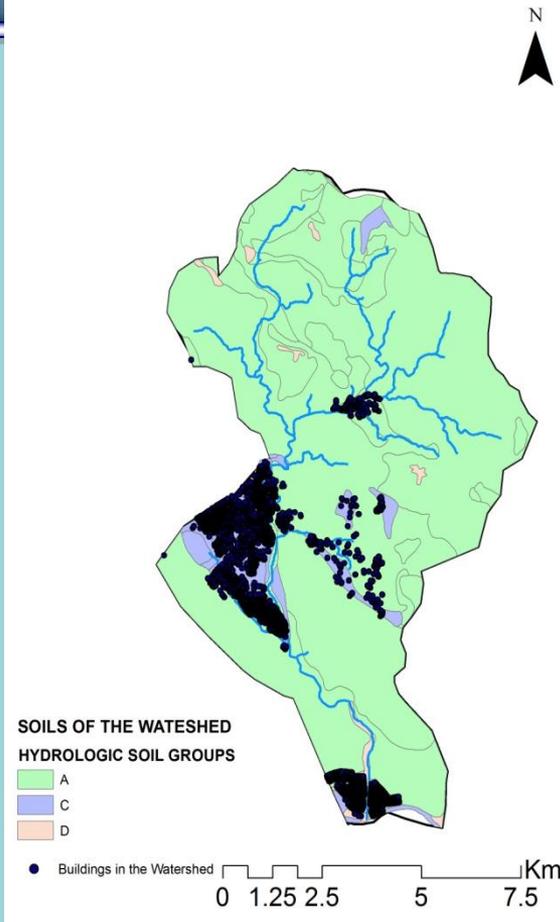
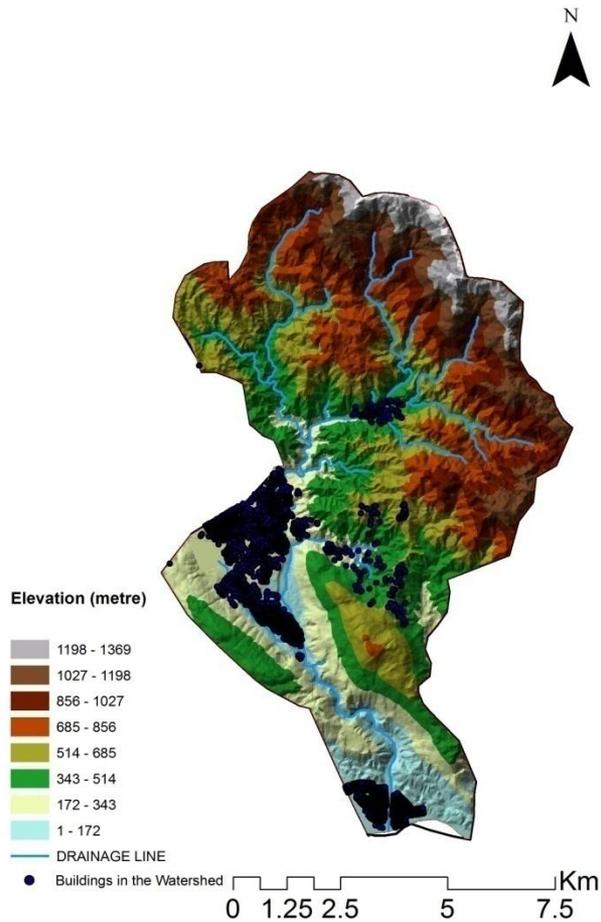
The Hope River Watershed had repeated occurrences of flooding and associated debris flows resulting from heavy rains associated with Tropical storms and hurricanes.

Damages have resulted in collapse of bridges (at Kintyre and Harbour view and houses along the flood plain from hurricane Gustav (2008), tropical storm Nicole (2010). These are some of the recent damages.

Aims of the study on Hope River Watershed

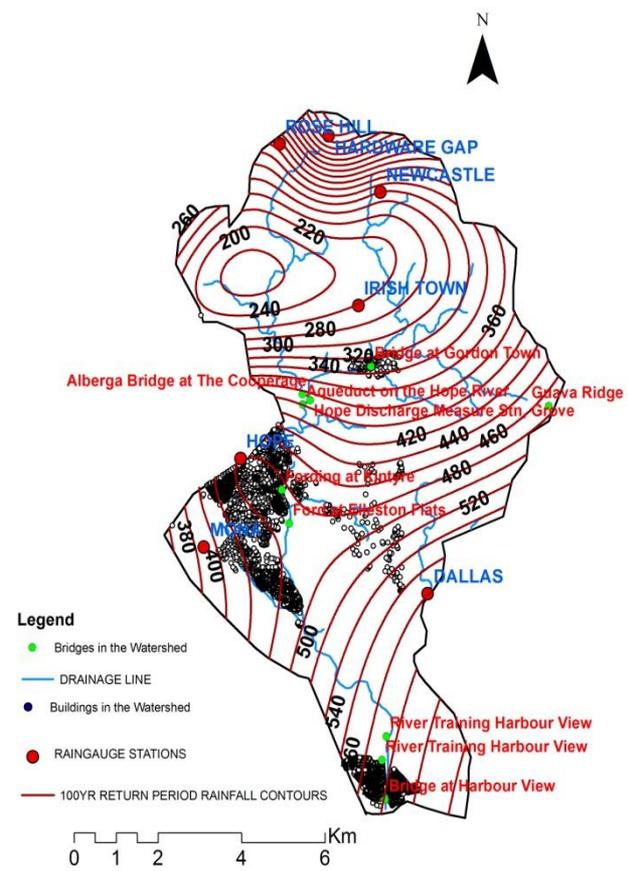
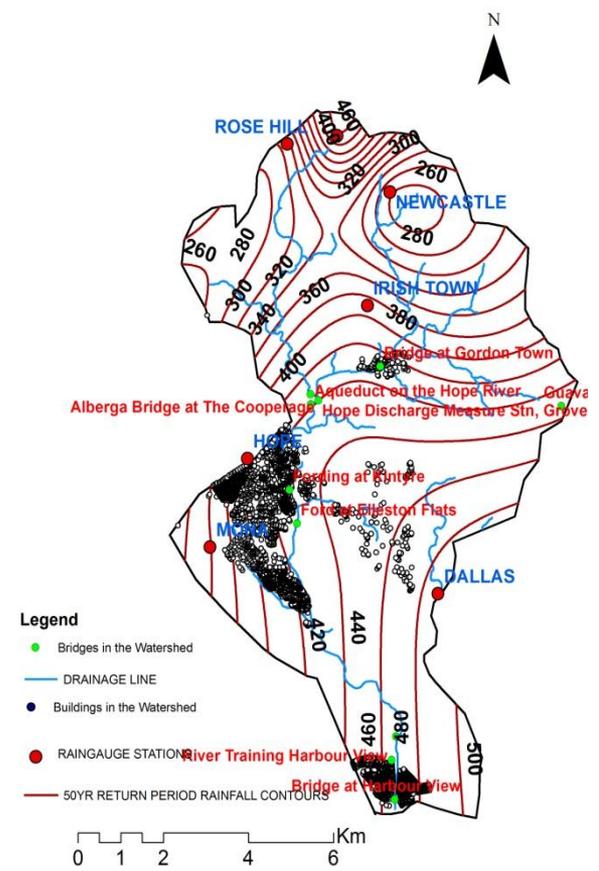
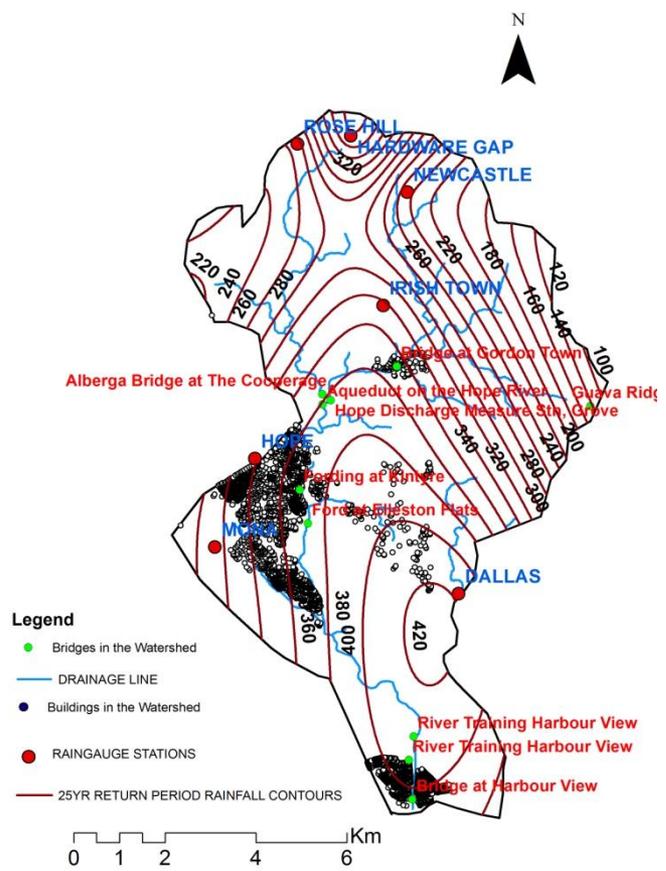
- **Assessment of damages done by flooding.**
- **Study on rainfall return periods and rainfall pattern for the watershed.**
- **Analysis of the flood return periods.**
- **Analysis of long term (1955-2010) discharge data from the Hope River.**
- **Flood plain maps for the watershed and highlighting vulnerable zones.**

ELEVATION, LANDUSE AND SOIL TYPE OF THE WATERSHED



Elevation Map created from DEM of 6m horizontal and 1m vertical resolution. Note the buildings are located in the floodplains of the river and in the areas of low elevation. High elevation in the upper watersheds coupled with impermeable rocks results in less infiltration. Hence high run off and flooding in the downstream areas. Soil data shows buildings located in zones with moderate high runoff ie C.

Return Periods for Maximum 24hr rainfall for the watershed . (Data 1937-1985, Metservice of Jamaica)



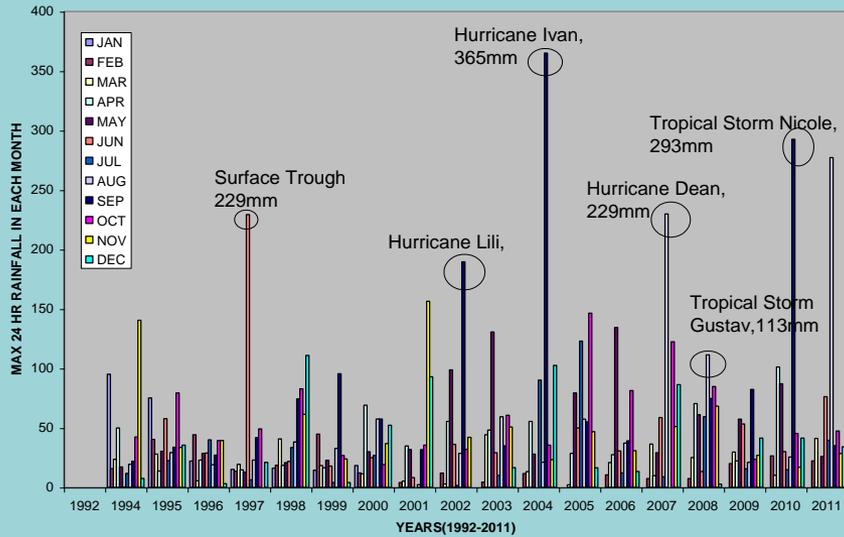
**25yr return period
rainfall contours
(mm)**

**50yr return period
rainfall contours
(mm)**

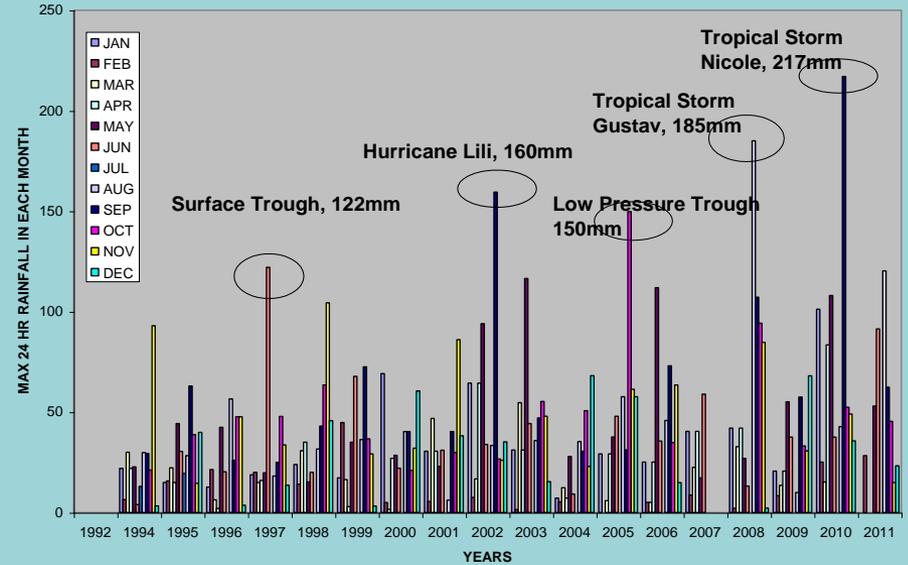
**100yr return period
rainfall contours
(mm)**

MAXIMUM RAINFALL FOR SOME STATIONS OF THE HOPE WATERSHED AND ASSOCIATED TROPICAL STORMS, HURRICANES AND TROUGHS.

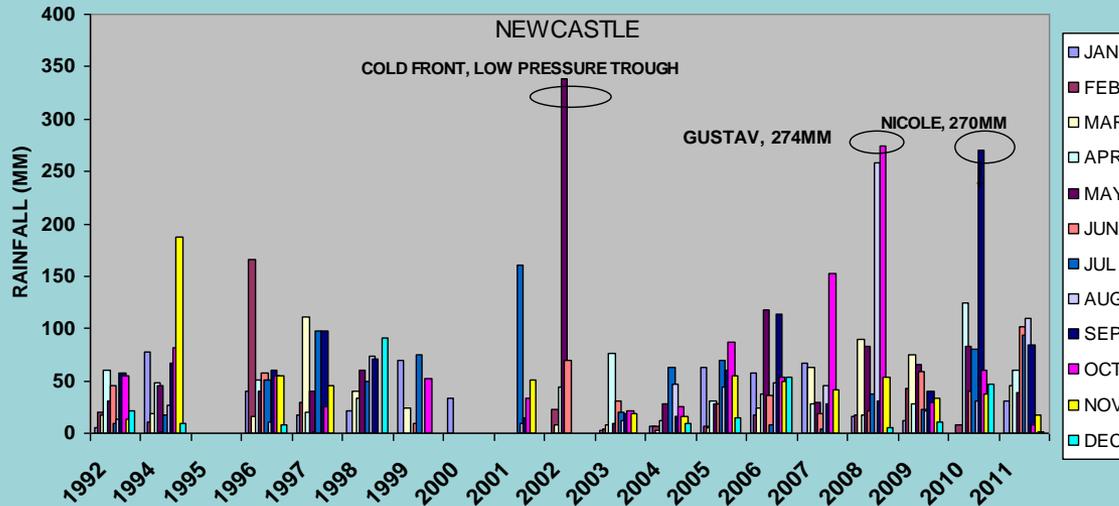
IRISH TOWN



JACKS HILL

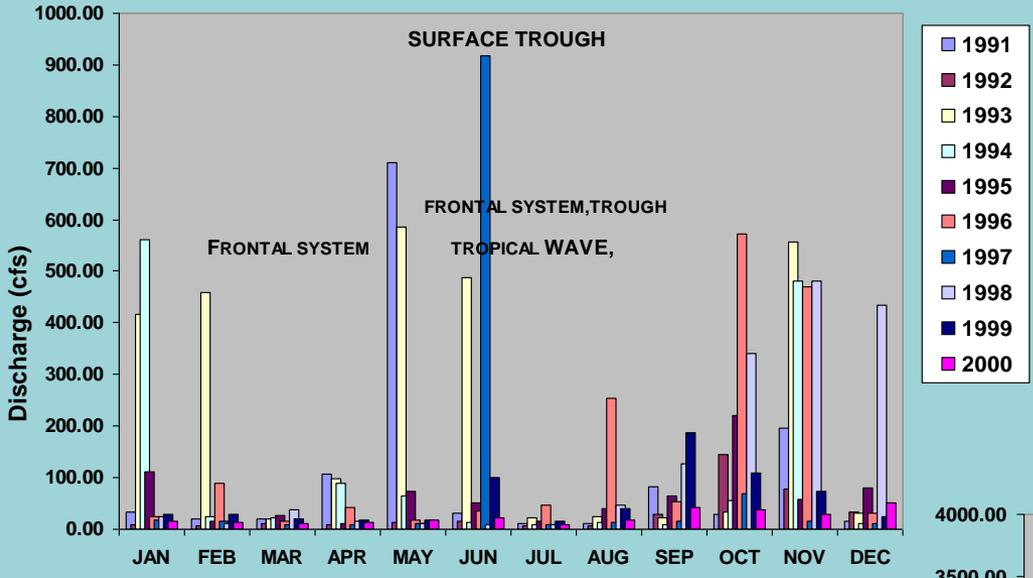


NEWCASTLE

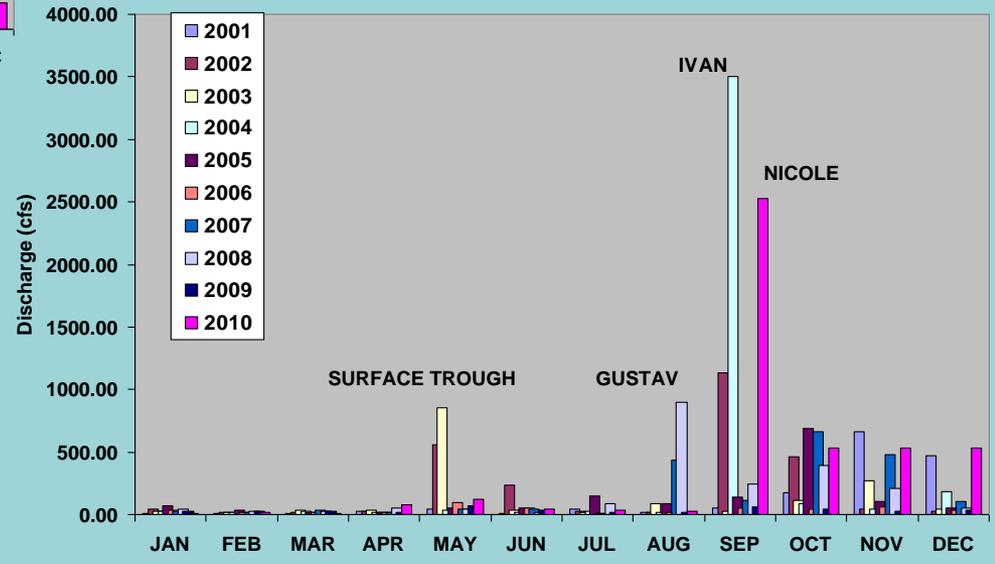


MAXIMUM PEAK DISCHARGES FOR THE HOPE RIVER MEASURED AT THE HOPE DISCHARGE STATION AT GROVE AND ASSOCIATED CAUSES.

Maximum Daily Discharge(1991-2000)

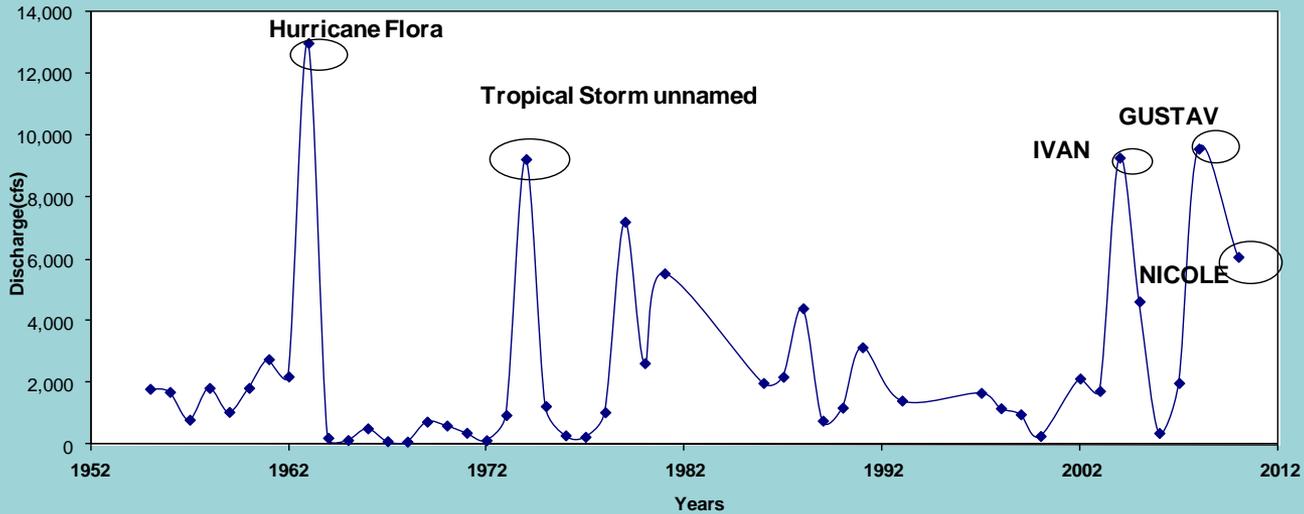


Maximum Daily Discharge (2001-2010)

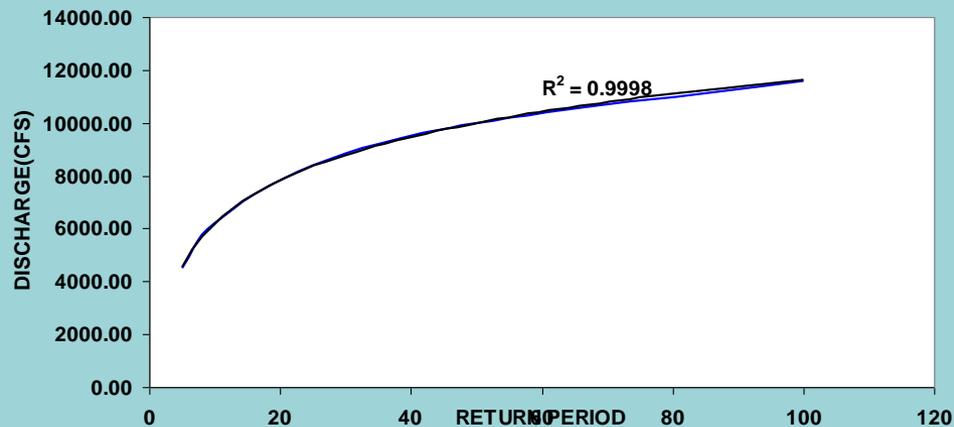


PEAK FLOOD DISCHARGES AND LONG TERM MAXIMUM DISCHARGE FROM HOPE DISCHARGE STATION. RETURN PERIOD OF FLOOD PEAK FLOWS USING GUMBEL METHOD (MOMENT OF MEANS)

Instantaneous Peaks



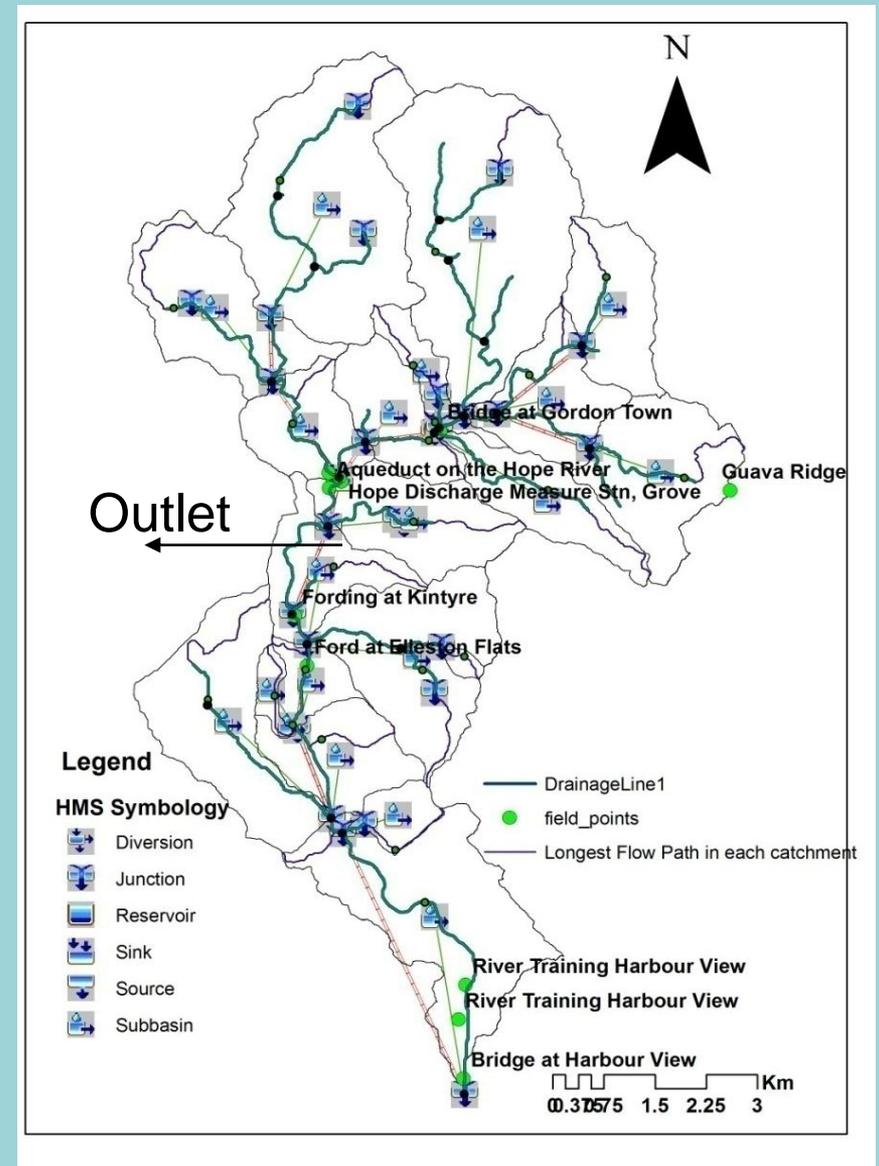
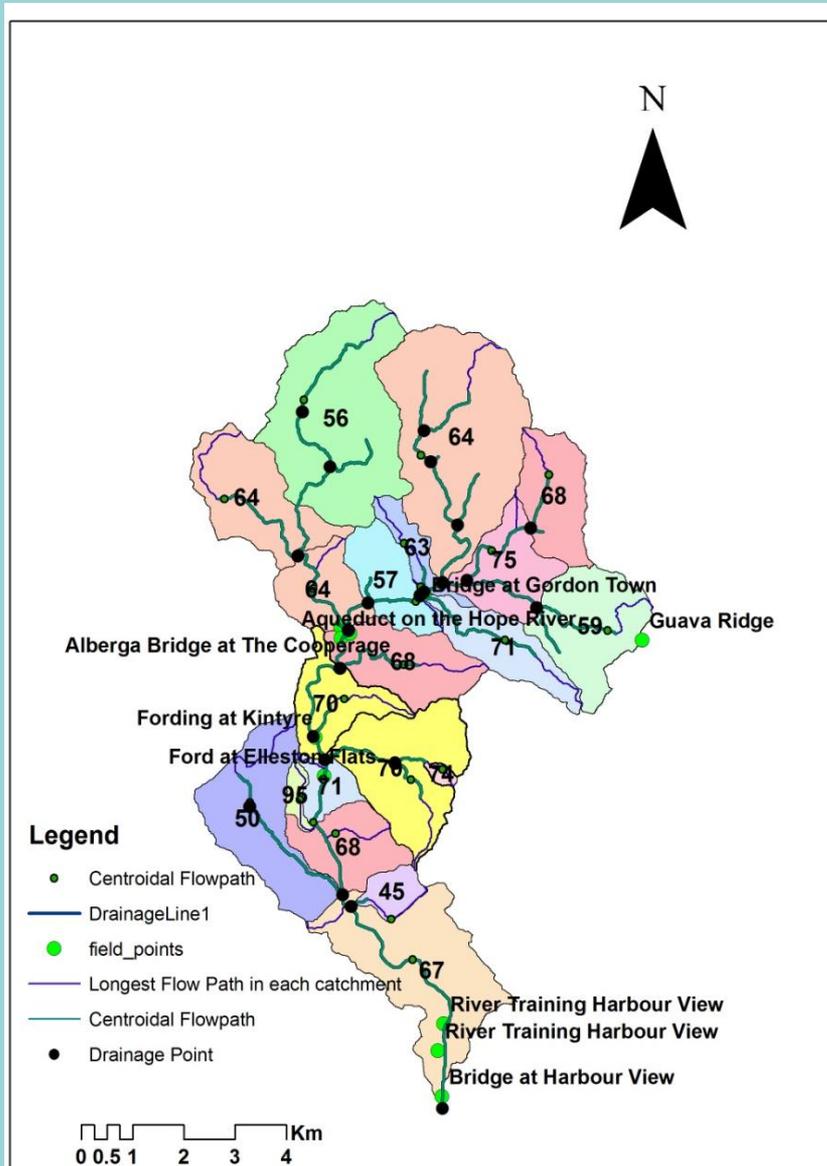
DISCHARGE VS RETURN PERIOD



Return Period	Flow(cfs)
5	4495.73
10	6219.23
25	8382.22
50	9990.82
100	11599.65

Catchments, flowlengths, curve number for the watershed created in GEOHMS

Basin model with junctions, sub-basins, reaches and outlets as in HEC-HMS



SIMULATION OF THE RAINFALL – RUNOFF MODEL IN HEC HMS AND CALIBRATION WITH OBSERVED DISCHARGE DATA

HEC HMS was run with 2 meteorological model

- 5 minute rainfall intensity data for Tropical Storm Nicole measured at the Hope discharge station
- 5 minute rainfall intensity data from raingauge at Hope discharge station for the month of June 2011.

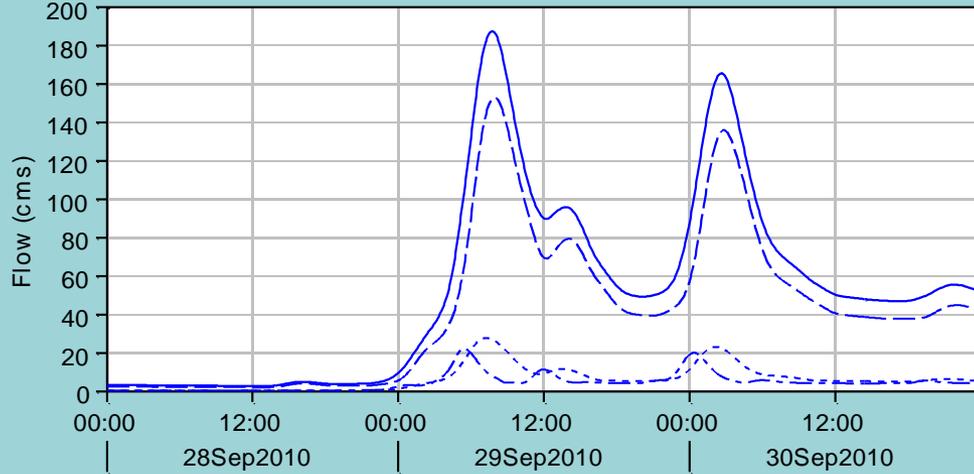
The SCS loss method and basin lag were used as other input data. Basin lag was determined using SCS formula:

$TL = L^{0.8}(SCN+1)^{0.7}/1900*Y^{0.5}$ where, L= length of the longest flowpath, $SCN = (1000/CN - 10)$, Y = Slope .

Model calibrated with observed discharge data from hope discharge station.

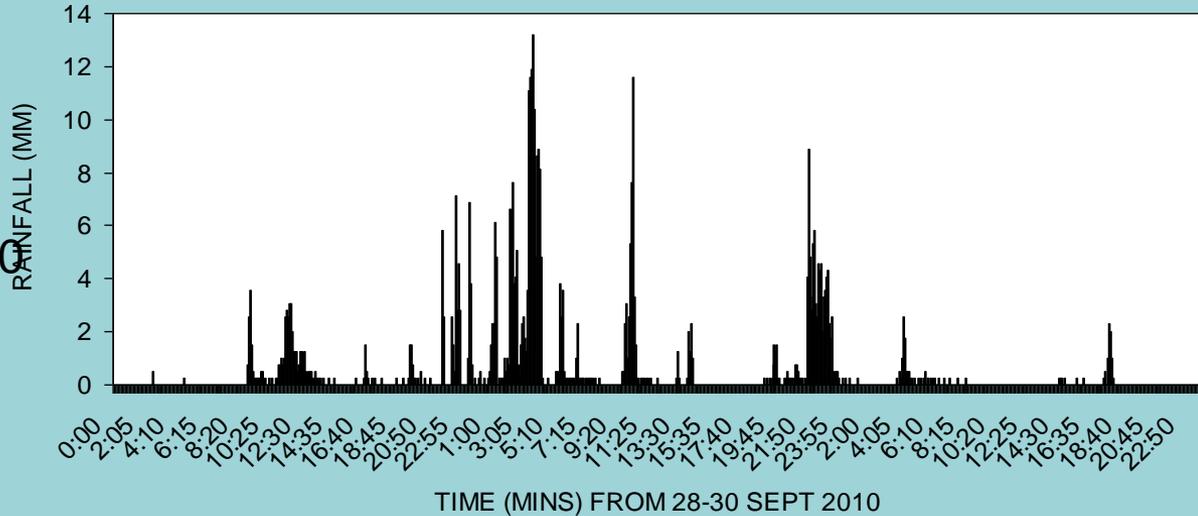
RESULTS FROM HEC HMS

Sink "Outlet62" Results for Run "NICOLE_Muskingum"

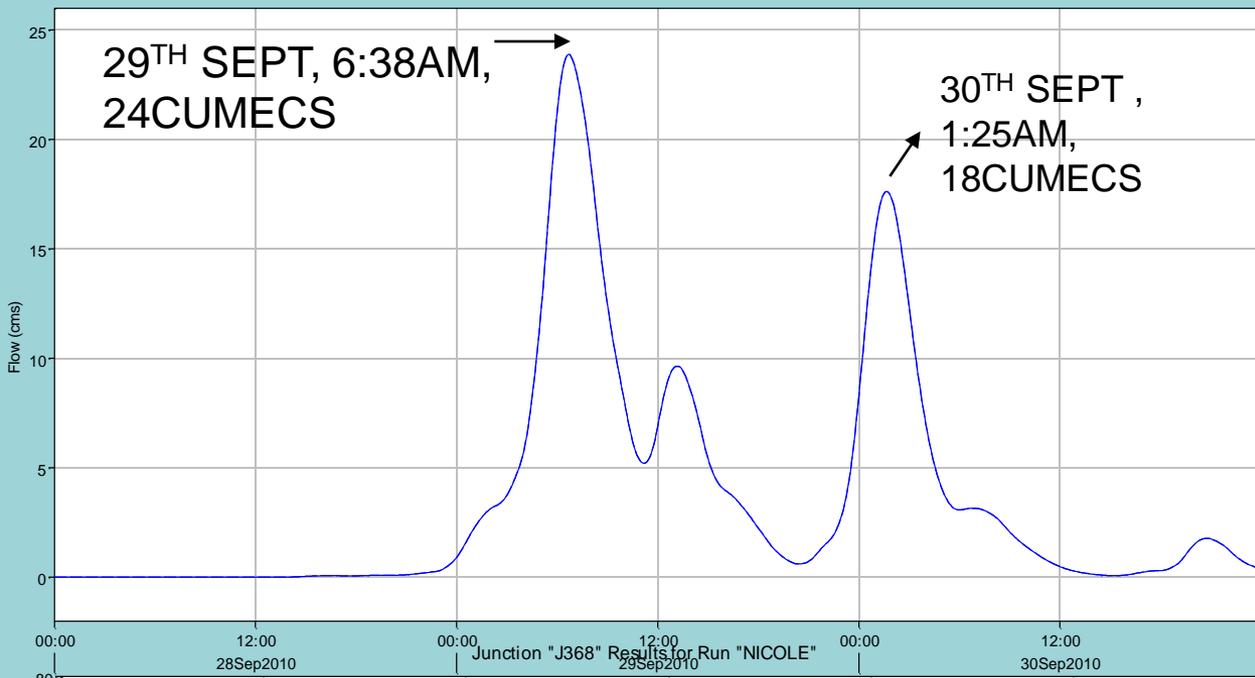


- Run: NICOLE_MUSKINGUM Element: OUTLET62 Result: Observed Flow
- Run: NICOLE_MUSKINGUM Element: OUTLET62 Result: Outflow
- Run: NICOLE_Muskingum Element: R1690 Result: Outflow
- - - Run: NICOLE_Muskingum Element: R1680 Result: Outflow
- - - Run: NICOLE_Muskingum Element: W1130 Result: Outflow

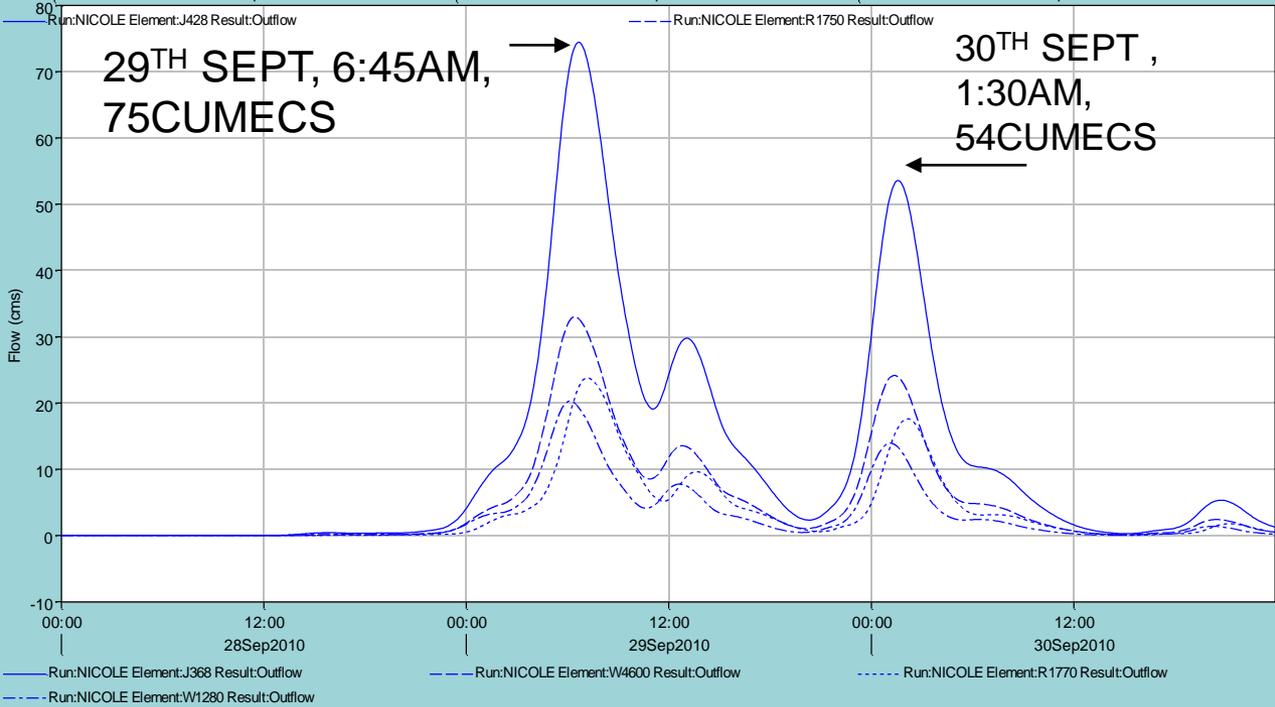
5 MINUTE INTENSITY RAINFALL DATA FOR TROPICAL STORM NICOLE



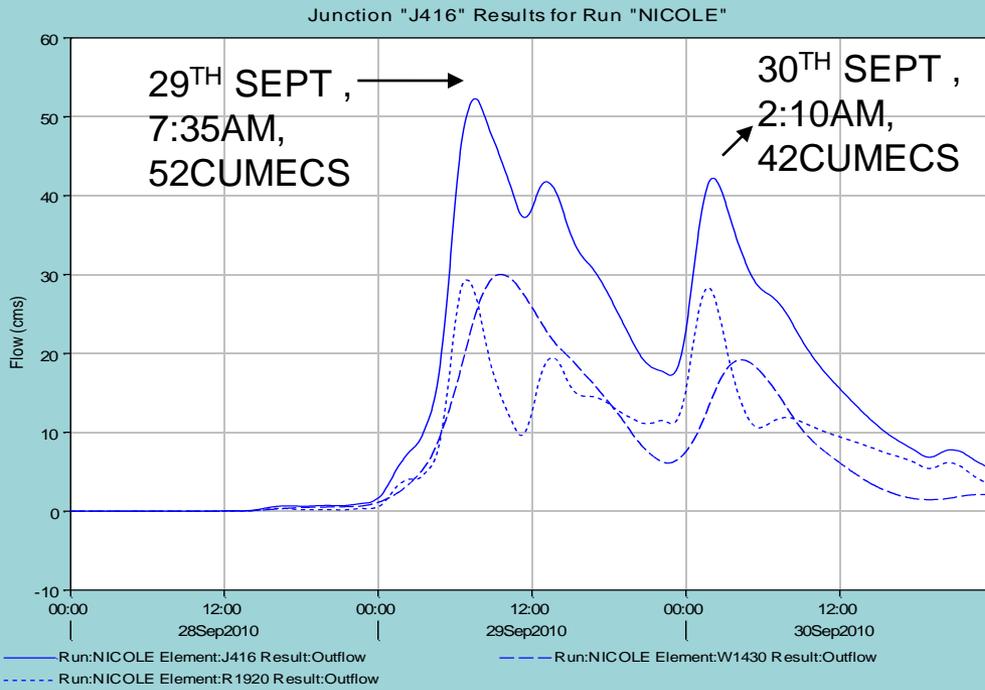
Peak rainfall = 13.2MM AT 3:40 AM , 29TH SEPT



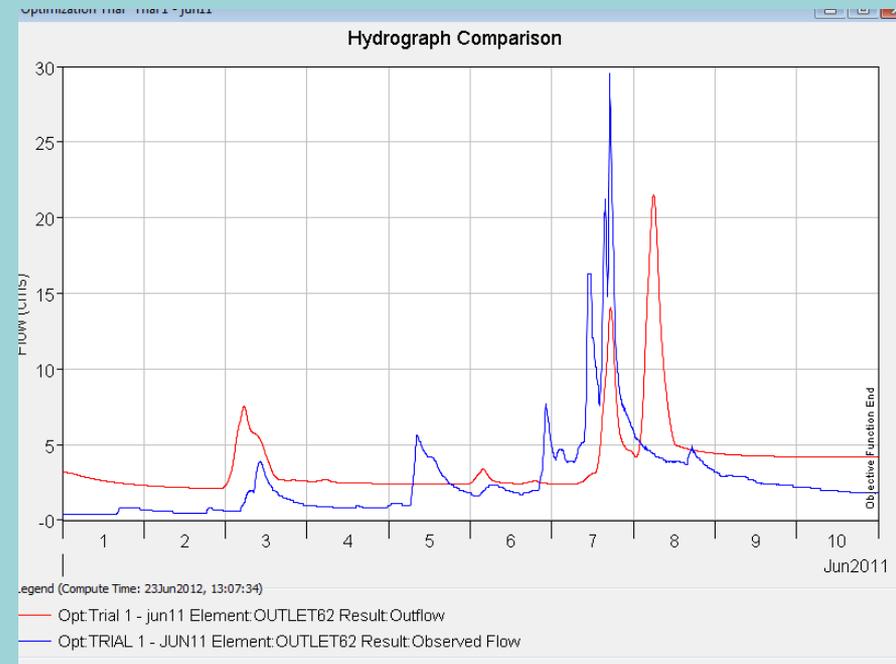
Simulated peak discharges at junction of flow nodes at the fording at Kintyre.



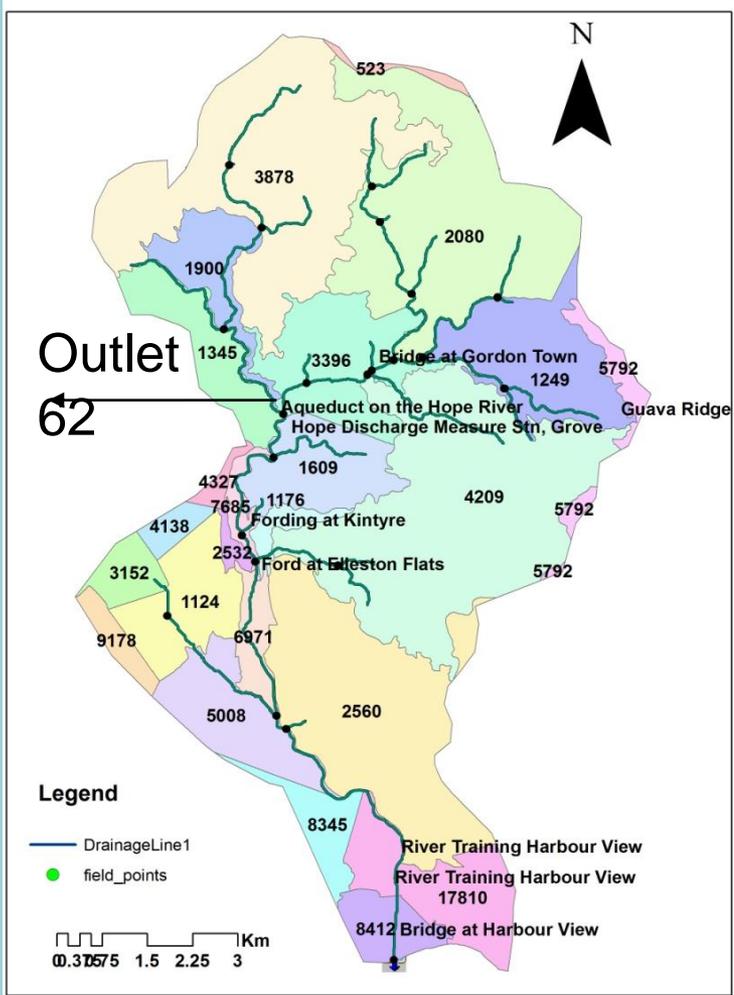
Simulated discharge at the junction of flow nodes immediate upstream (~330m) north of fording at Elleston Flats.



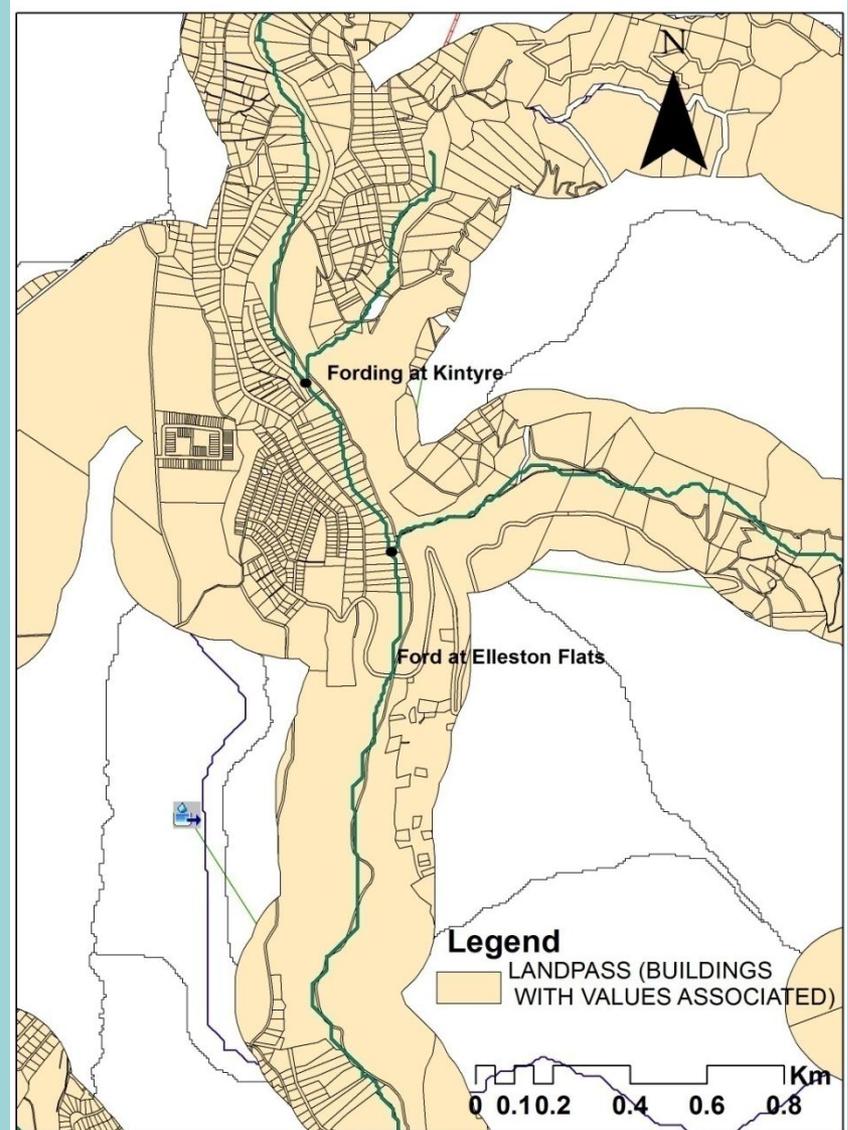
Simulated discharge at the junction node near the mouth of the river at harbour view.



Simulated vs observed hydrograph for the hope discharge station at grove which is marked as outlet 62 in the hec hms model. This is the outlet of flow from the sub-basins of the upper part of the watershed.



Population of the Hope River watershed. The vulnerable areas i.e., Kintyre, Elleston Flats/August Town which are in the high peak flow zone have a population of 1609, 1176, 2532 respectively. Population of Harbour View ranging from 8412-17810. Outlet 62 = peak of 173CFS(Nicole).



Buildings within a distance of 200m from the river for two of the vulnerable communities. Approx range of value for houses left of Kintyre ~ 450000 to 550000

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- ❖ World Bank For Funding The Research Work
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- ❖ National Land Agency, Jamaica.
- ❖ Dept of Geography and Geology, UWI Mona, Jamaica.
- ❖ Dept of Physics, UWI Mona, Jamaica.
- ❖ University of the French West Indies in Guadeloupe.

THANK YOU